### **BOSTON REGION METROPOLITAN PLANNING ORGANIZATION**



Stephanie Pollack, MassDOT Secretary and CEO and MPO Chair Karl H. Quackenbush, Executive Director, MPO Staff

## TECHNICAL MEMORANDUM

- DATE: December 6, 2018
- TO: James Kupfer and Donald DiMartino, Town of Bellingham
- FROM: Seth Asante and Benjamin Erban, MPO Staff
- RE: Redesign of Hartford Avenue and Maple Street Intersection

This memorandum summarizes the analyses and improvement strategies for the intersection of Hartford Avenue (Route 126) and Maple Street in Bellingham.

The memorandum contains the following sections:

- 1. Study Background
- 2. Existing Facilities and Land Uses
- 3. Issues and Concerns
- 4. Existing Traffic Conditions
- 5. Crash Data Analysis
- 6. Existing Traffic Operations
- 7. Improvement Alternatives
- 8. Conclusions and Next Steps

It also includes technical appendices that contain data and methods applied in the study.

#### 1 STUDY BACKGROUND

The purpose of the Safety and Operations Analyses at Selected Intersections study is to examine safety, operations, and mobility issues at major intersections in the Boston Region Metropolitan Planning Organization (MPO) region's arterial highways—areas where many crashes occur, that experience congestion during peak traffic periods, or are in need of improvements to accommodate heavy vehicles (buses and trucks) or nonnotarized transportation (bicyclists and pedestrians). For the past 10 years, the MPO has conducted these planning studies, which have been well received by the municipalities in the region. These studies give communities an opportunity to look at the needs of the select locations, starting at the conceptual level, before they commit funds for design and engineering. Eventually, if the project qualifies for federal funds, the study's documentation also is useful to the Massachusetts Department of Transportation (MassDOT). These studies support the MPO's visions and goals, which include increasing transportation safety, maintaining the transportation system, advancing mobility, and reducing congestion.

State Transportation Building • Ten Park Plaza, Suite 2150 • Boston, MA 02116-3968 Tel. (857) 702-3700 • Fax (617) 570-9192 • TTY (617) 570-9193 • www.bostonmpo.org Following a selection process based on safety conditions, congested conditions, multimodal significance, regional significance, regional equity, and implementation potential, the following two locations from a short list of 20 intersections were approved for study by the MPO.<sup>1,2,3,4,5,6,7</sup>

- 1. Hartford Avenue (Route 126) at Maple Street in Bellingham
- 2. Main Street (Route 1A) at Arbor Street, Monument Avenue, and Cherry Street in Wenham

The location in Bellingham was selected because the intersection at Hartford Avenue and Maple Street carries a high proportion of truck traffic and is undersized to accommodate large commercial vehicles safely and efficiently. The intersection is just one-half mile east of the interchange of Interstate 495 and Route 126, where a number of large commercial uses exist. In addition, the area along Maple Street is zoned for industrial uses and currently home to several businesses and industrial properties (a power plant, multiple warehouses exceeding 600,000 square feet of space, and large-scale mulch and lumber hauling and production). The Town of Bellingham recognizes the need to upgrade the intersection in order to maintain the industrial uses and unlock future investment potential of the surrounding area, which is projected to grow in the future. Figure 1 shows the location of the intersection and the surrounding roadways.

<sup>&</sup>lt;sup>1</sup> Safety Conditions: Location has a higher-than-average crash rate for its functional class, contains a Highway Safety Improvement Program (HSIP)-eligible crash cluster, contains a top-200 high crash location, or has a significant number of pedestrian and bicycle crashes (two or more per mile).

<sup>&</sup>lt;sup>2</sup> Congested Conditions: Travel time index is at least 1.3.

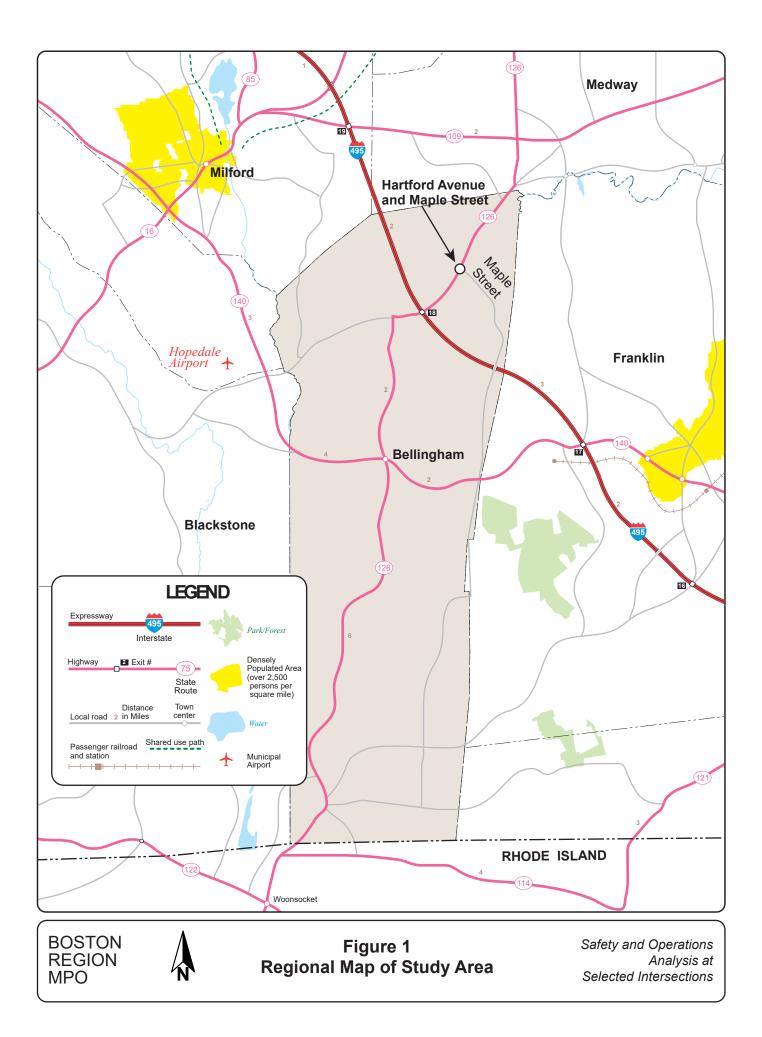
<sup>&</sup>lt;sup>3</sup> Multimodal Significance: Location carries bus route(s), is adjacent to a transit stop or station; supports bicycle or pedestrian activities or has an implementation project to support one or more of these activities; has need to accommodate pedestrians and bicyclists and improve transit; or high truck traffic serving regional commerce.

<sup>&</sup>lt;sup>4</sup> Regional Significance: Location is in National Highway System; carries a significant portion of regional traffic (ADT >20,000); lies within 0.5 miles of Environmental Justice transportation analysis areas or zones; or is essential for the region's economic, cultural, or recreational development.

<sup>&</sup>lt;sup>5</sup> Regional Equity: That is, it was important not to select 1) more than one location in a subregion and 2) a location in same subregion as in the preceding cycle of this study.

<sup>&</sup>lt;sup>6</sup> Implementation Potential: Location is proposed or endorsed by its roadway administrative agency (agencies); proposed or endorsed by its subregion and is a priority for that subregion; or has strong support from other stakeholders.

<sup>&</sup>lt;sup>7</sup> Safety and Operations Analyses at Selected Intersections: Federal Fiscal Year 2018, Technical Memorandum to the Boston Region Metropolitan Planning Organization. Seth Asante and Chen-Yuan Wang, January 18, 2018.



#### 1.1 Public Participation

MPO staff discussed the safety and operations issues at the intersection and the scope of work for the study with the Town of Bellingham, which expressed interest and willingness to participate in the study. An advisory task force—composed of representatives from the Town of Bellingham, MassDOT District 3, and MassDOT Office of Transportation Planning (OTP)—was established to guide this study. MPO staff met with the task force twice. The first meeting focused on the work scope and existing problems. In the second meeting, MPO staff presented the existing conditions, analyses, proposed improvements, and received advice from the task force members. This report reflects the task force's feedback. Appendix A includes a list of task force members, information about the selection process, and comments about the study.

#### 2 EXISTING FACILITIES AND LAND USES

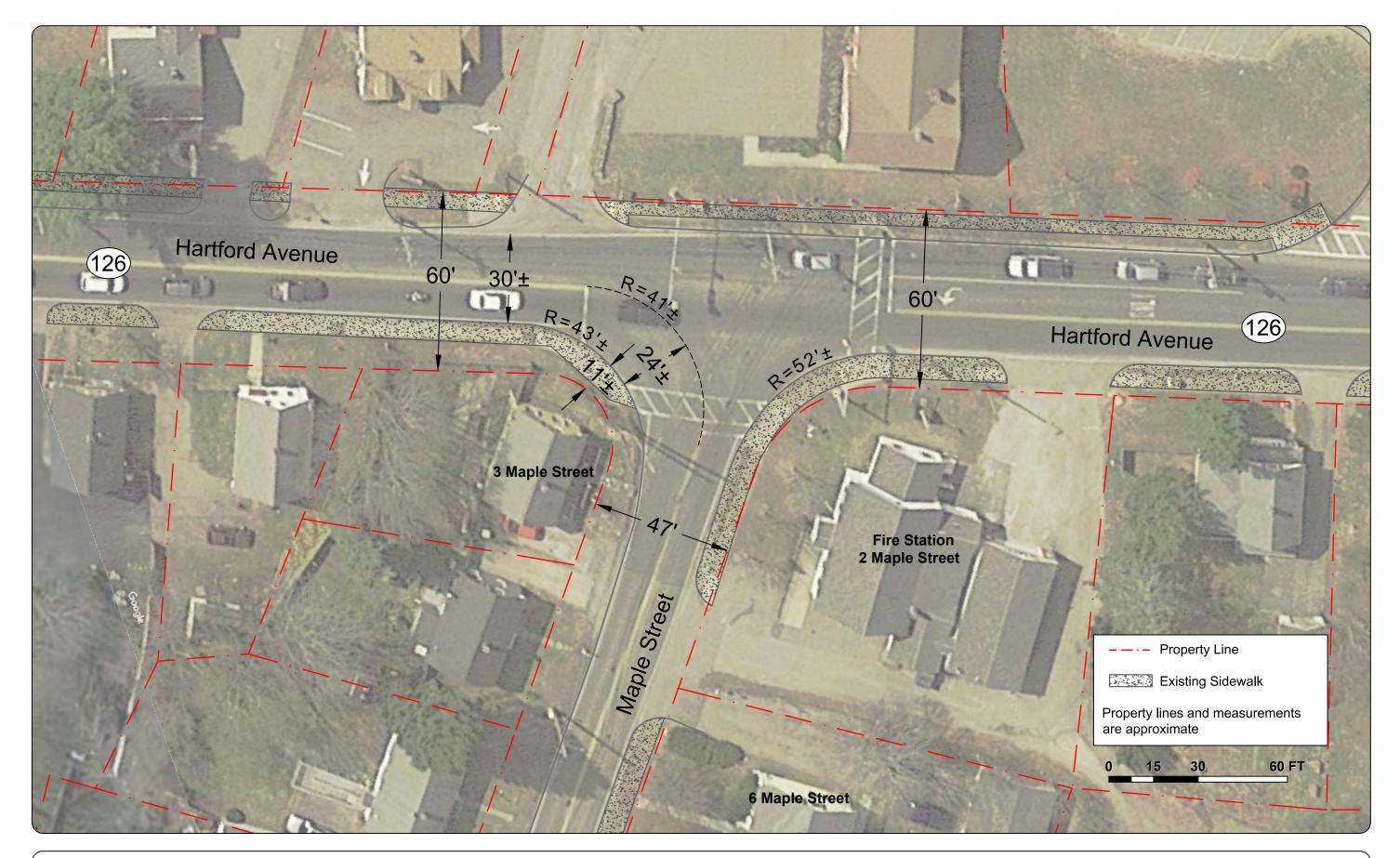
#### 2.1 Roadway and Geometry

The study intersection is located in the northeast corner of the Town of Bellingham, approximately one-half mile north of Exit 18 off of I-495. Hartford Avenue (Route 126) intersects with the northern terminus of Maple Street at a traffic signal. Route 126 is classified as a principal arterial (other) and is a townaccepted roadway despite being a numbered route. Both streets are two-lane, two-way roadways. The right-of-way of Hartford Avenue is 60 feet and the rightof-way of Maple Street is about 47 feet. Figure 2 shows the existing intersection geometry.

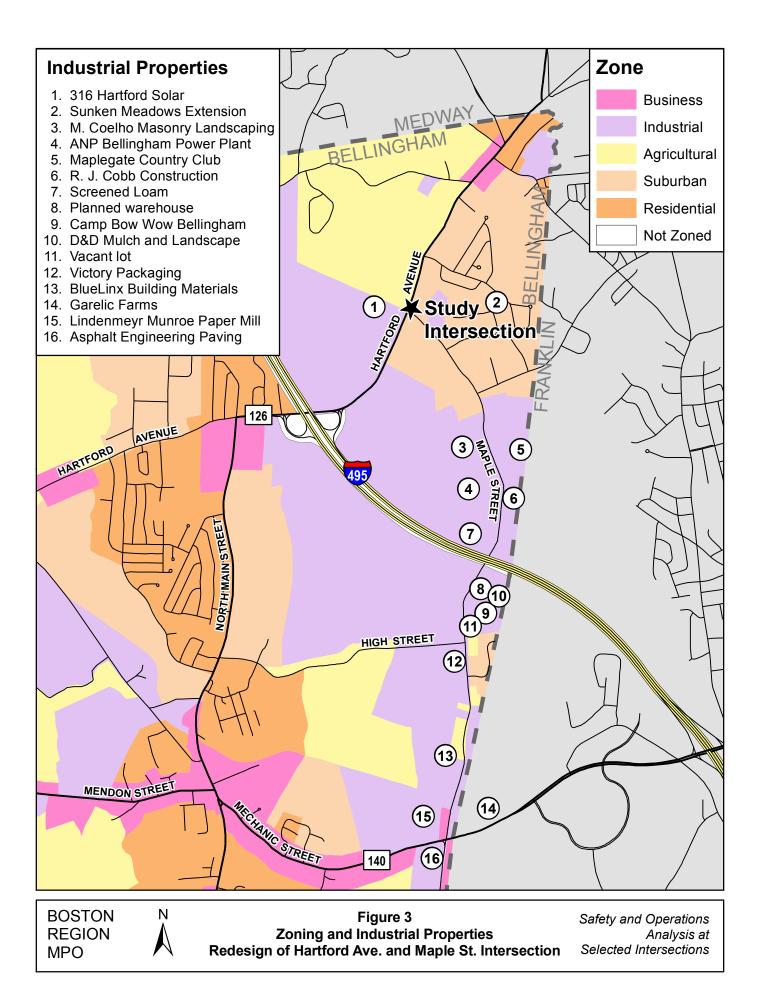
#### 2.2 Land Uses

The property on the northeast corner of the intersection is owned by the Town of Bellingham and is currently used as an auxiliary garage for the Bellingham Fire Department. The property on the southeast corner of the intersection, 3 Maple Street, is a private residence. On the west side of Hartford Avenue are several small businesses, as well as Stall Brook Elementary School and Bellingham Early Childhood Center. The school has 19 classrooms and serves 325 students. Stall Brook School may be accessed via an unsignalized driveway that enters the study intersection, although traffic counts show that the vast majority of school traffic uses a second unsignalized driveway about 200 feet north of the intersection.

While the properties adjacent to the intersection are mostly residential and commercial, the surrounding area—particularly further down Maple Street—has a significant amount of land zoned industrial. Figure 3 gives a map of the zoning in the vicinity of the intersection and notes several existing or planned industrial businesses.







#### 3 ISSUES AND CONCERNS

The primary issue at this intersection is truck maneuvers to and from Maple Street. Heavy vehicles have difficulty making left and right turns in the intersection. Taking a right from Hartford Avenue northbound onto Maple Street is the shortest route to access Maple Street from I-495, although the turn onto Maple Street is difficult for trucks because of a substandard curb radius. Figure 4 shows the telephone pole on the southeast corner of the intersection, which is frequently damaged by trucks making a right turn onto Maple Street from Route 126 northbound. The left turn out of Maple Street is also difficult, and heavy vehicles leaving Maple Street properties are technically required to head south to Mechanic Street (Route 140) instead of using the Hartford Avenue and Maple Street intersection, which is an inconvenience. Most of the complaints from residents about this intersection relate to the turning of large heavy vehicles.



Figure 4 Telephone Pole in front of 3 Maple Street

Maple Street already sees a high volume of truck traffic as a consequence of the existing industrial properties (shown in Figure 3), and the corridor is primed for further growth in the near future. Many of the industrial-zoned properties along Maple Street are either undeveloped (woodland or swamp) or underdeveloped properties where new businesses have expressed interest. Planned industrial projects include a new 450,000 square foot industrial warehouse currently under construction across the street from a dog daycare center, Camp Bow Wow, as well as a second warehouse further down the corridor. Additionally, the Maplegate Country Club, which spans the border with Franklin, recently changed ownership and potentially could be redeveloped, freeing up a large area for industrial development.

This area is in high demand because it is one of the few suitable sites for warehouses near I-495. In addition, its location near the Massachusetts border makes it a good stopover point for redistributing loads to meet weight regulations in Connecticut and Rhode Island. Several mulch distributors on Maple Street currently take advantage of this strategic placement.

Addressing issues at the intersection of Hartford Avenue and Maple Street is of particular interest because the intersection is the limiting factor for further development along Maple Street. The Town of Bellingham has previously made several investments in Maple Street. A redesign of the intersection at the other end of Maple Street (with Route 140) is already planned, with design work provided by the traffic consultant BETA Group and funding from both MassWorks grants and a private developer. Bellingham has spent \$1.0 million on Maple Street itself to improve drainage, widen shoulders, and repair wear and tear from heavy vehicle traffic. Kleinfelder was the design contractor for this work.

In summary, addressing issues caused by heavy vehicles at the study intersection will improve safety for residents and drivers, improve access and convenience for businesses, and help the town meet its vision of a profitable industrial corridor along Maple Street.

#### 4 EXISTING TRAFFIC CONDITIONS

#### 4.1 Daily Traffic Volumes

MassDOT Highway Division's Traffic Data Collection section conducted automatic traffic recorder (ATR) counts over a two-day period from Monday February 12, 2018, to Wednesday February 14, 2018. The counts continuously collect traffic volumes, speeds, and classifications over the collection period, and are used to determine the average weekday traffic (AWDT) of a roadway. The counts were performed at three locations adjacent to the study intersection; however, a malfunction with the equipment placed on Route 126 north of Maple Street caused these data to be unusable.

Figure 5 presents a summary of ATR traffic data. Route 126 carried 18,200 vehicles per day and Maple Street carried 11,500 vehicles per day. The average speeds of travel in the vicinity of the intersection were 30 miles per hour (mph) on Route 126 and 34 mph on Maple Street. In comparison, the posted speed limits on Route 126 and Maple Street are 35 mph and 30 mph, respectively. The directional split on both roadways was almost equal (50 percent of the daily traffic was recorded in each direction). Full details of the counts can be found in Appendix B.

#### 4.2 Turning Movement Volumes

MassDOT Highway Division's Traffic Data Collection Section also collected turning-movement counts (TMC) in the study area during January 2018, while schools were in session. MassDOT conducted the counts during the weekday AM peak travel period (6:00 AM–9:00 AM) and weekday PM peak travel period (2:00 PM–6:00 PM). The counts were conducted both at the study intersection of Route 126 at Maple Street and at the driveway of Stall Brook School. Heavy vehicles such as school buses, transit buses, and trucks were counted separately. Pedestrian and bicycle counts were conducted simultaneously with the TMCs.

Figure 6 shows the peak-hour turning movement, pedestrian, and bicycle volumes in the study area. The peak hours were 7:00 AM–8:00 AM for the morning peak and 4:30 PM–5:30 PM for the evening peak. There were 19 total pedestrian crossing events observed during the seven-hour observation interval, although only three of those took place during the peak hours. Eight were counted at the beginning of the school day for Stall Brook School (8:30 AM) and five at the end of the school day (2:30 PM). No cyclists were observed.

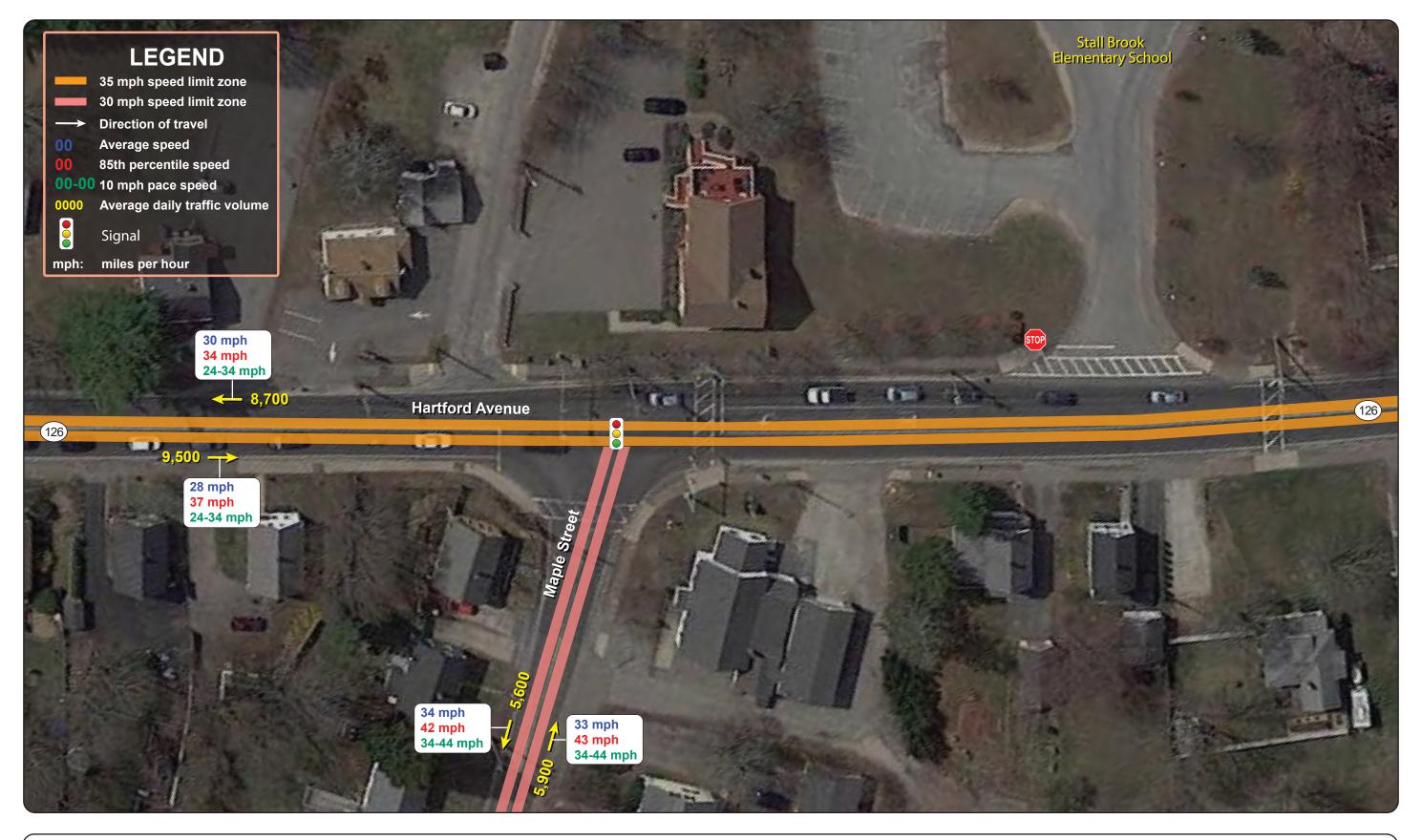




Figure 5 Average Daily Traffic Volumes, Speed Regulations, and Estimated 85th Percentile Speeds Redesign of Hartford Avenue and Maple Street Intersection

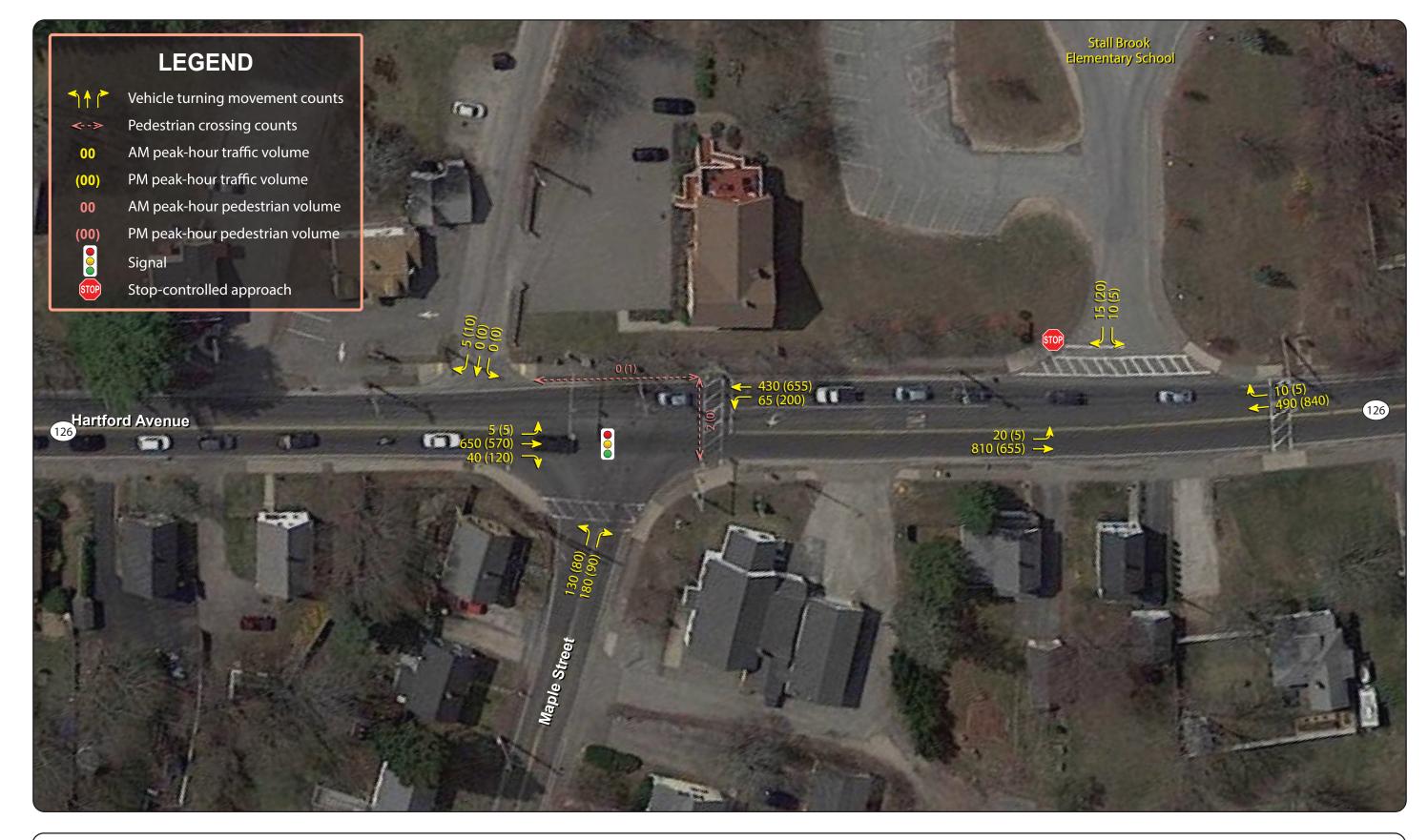




Figure 6 Weekday Peak-Hour Traffic and Pedestrian Volumes Redesign of Hartford Avenue and Maple Street Intersection

#### 4.3 Heavy Vehicle Traffic

Because heavy vehicles were important to the project objective, MPO staff also investigated the patterns of heavy vehicle traffic through the study intersection. Figure 7 shows the hourly distribution of heavy vehicle traffic, based on ATR classification data. On Route 126, heavy vehicle traffic averages 4 percent of traffic and is highest during the morning peak period. On Maple Street, heavy vehicle traffic averages 3 percent of traffic and is highest overnight.

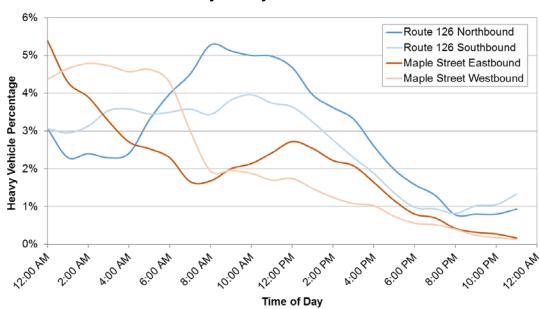


Figure 7 Hourly Heavy Vehicle Traffic

Heavy vehicles counted in the TMCs were also compared. Over the course of the seven-hour observation interval, 373 total heavy vehicles were recorded, although 80 percent of those passed north south through the intersection along Route 126 without performing a turning movement to access Maple Street. There were 23 heavy vehicles that entered Maple Street from Route 126 southbound and 11 that entered from Route 126 northbound. The pattern of heavy vehicles leaving Maple Street was almost symmetrical, with 23 turning onto Route 126 northbound and 14 turning onto Route 126 southbound.

The observed heavy vehicle volumes indicate that high numbers of trucks are not the primary issue, but rather safety and driver comfort. However, it is likely that more drivers will choose to use the intersection if it is changed to better accommodate their needs.

#### 5 CRASH DATA ANALYSIS

#### 5.1 Collision Trends

Twenty crashes were recorded by the Bellingham Police Department over the five-year period between 2011 and 2015. Table 1 breaks these crashes down by type of collision, severity, and factors that may have influenced the crash. It also compares the crash rate (crashes per million entering vehicles) with the District 3 average for signalized intersections. The crash rate calculations are in Appendix C.

Crash Variable	Number of Crashes
Crash Severity	_
Nonfatal injury	3
Property damage only	17
Manner of Collision	_
Angle	2
Head-on	1
Rear-end	16
Sideswipe, opposite direction	0
Sideswipe, same direction	0
Single vehicle crash	0
Unknown	1
Road Surface Conditions	_
Dry	12
Snow/ice	1
Wet	7
Ambient Light Conditions	_
Dark–lighted roadway	5
Daylight	14
Dusk	1
Other	0
Weather Conditions	_
Clear	12
Cloudy	2 5
Rain	5
Snow	1
Bicyclists and Pedestrians Involved	
Bicyclist	0
Pedestrian	0
Time Period	
Peak period	14
Off-peak period	6
Total Crashes	20
Five-year average (rounded)	4
Crash rate (calculated)	0.61
Crash rate (MassDOT District 3)	0.89

Table 12011–15 Crash Summary and Crash Rates

Some highlights of the data include:

- 7 crashes (35 percent) occurred on wet roadways
- 16 crashes (80 percent) were rear-end collisions
- There were no fatal crashes, and only 3 crashes (15 percent) resulted in injury
- There were no crashes involving pedestrians or cyclists

#### 5.2 Collision Diagram

MPO staff also prepared a collision diagram for the study intersection to examine crash patterns. Police reports from the Bellingham Police Department were obtained for the years 2011–15. Figure 8 shows the collision diagram. The index numbers in the collision diagram may be used to cross-reference the crash records in Appendix C. The index numbers with circles around them refer to injury or fatal crashes, depending on the thickness of the circle line.

#### 5.3 Safety Analysis

After analyzing the collision data, MPO staff concluded from the following information that safety was not the primary issue at the study intersection.

- The crash rate at the intersection was 0.61 crashes per million entering vehicles, which was lower than the 2016 MassDOT District 3 average for signalized intersections.
- Apart from the high proportion of crashes occurring in wet conditions, there were no notable trends revealed by the collision diagram, crash statistics, or police narratives. The majority of collisions were rear-ends caused by driver inattention.
- The injury rate was low, and there were no crashes involving pedestrians or cyclists.
- The intersection is not a cluster on the Statewide Top-200 Intersection Crash List for either 2012–14 or 2013–15, making it ineligible to receive funding through MassDOT's Highway Safety Improvement Program (HSIP).

The design alternatives presented in this study will nonetheless seek to improve driver and pedestrian safety where possible by bringing the intersection up to MassDOT design standards.

			The set of	Stall Brook Elementary School
Moving vehicle     Action     Moving vehicle	<ul><li>→□ Parked vehicle</li><li>→□ Fixed object</li></ul>	→← Head on →	Sideswipe	
	→ðð Bicycle → <i>≓</i> Animal	Rear end	Got of control	Inju
BOSTON REGION MPO		Collision Diagram: Mass	ure 8 DOT Crash Data 2011–15 and Maple Street Intersection	



#### 6 EXISTING TRAFFIC OPERATIONS

Using the data and information collected, MPO staff built a traffic analysis network (with Synchro)<sup>8</sup> for the AM and PM peak periods to assess the capacity and quality of traffic flow at the intersections. Staff conducted the analyses consistent with Highway Capacity Manual (HCM) methodologies.<sup>9</sup> The HCM methodology demonstrates the driving conditions at signalized and unsignalized intersections in terms of levels of service (LOS) ratings A through F. LOS A represents the best operating conditions (little to no delay), while LOS F represents the worst operating conditions (very long delay). LOS E represents operating conditions at capacity (limit of acceptable delay). Table 2 shows the control delays associated with each LOS for signalized and unsignalized intersections.

Level of Service	Signalized Intersections Control Delay (seconds per vehicle)	Unsignalized Intersections Control Delay (seconds per vehicle)
A	≤ 10	≤ 10
В	> 10–20	> 10–15
С	> 20–35	> 15–25
D	> 35–55	> 25–35
E	> 55–80	> 35–50
F	> 80	> 50

Table 2			
Levels of Service and Control Delays at Intersections			

Levels of Service of Existing Conditions							
Alternative/ Approach	Move- ment	AM LOS	AM Delay <sup>a</sup>	AM Queue <sup>b</sup>	PM LOS	PM Delay	PM Queue
Route 126 Northbound	LTR	С	25.6	430	С	31.2	#606
Route 126 Southbound	L	Α	6.0	20	В	16.1	99
Route 126 Southbound	TR	Α	7.1	123	А	8.2	238
Maple Street	LTR	E	59.5	#349	D	45.2	#191
Intersection Average	All	С	26.4	-	С	22.1	_

# Table 3Levels of Service of Existing Conditions

L = left turn. LOS = levels of service. R = right turn. T = straight through.

<sup>a</sup> Delay in seconds per vehicle.

<sup>b</sup> 95th percentile queue length in feet.

Notes:

# = the 95th percentile volume exceeds capacity.

Source: Central Transportation Planning Staff.

Table 3 presents peak-hour performance in terms of LOS, delay, and queues for existing conditions. The intersection operates near the top of LOS C conditions

<sup>&</sup>lt;sup>8</sup> Trafficware Inc., Synchro Studio 9, Synchro plus SimTraffic, Build 914, Sugar Land, Texas.

<sup>&</sup>lt;sup>9</sup> Highway Capacity Manual, HCM 2010, Volume 3: Interrupted Flow, Transportation Research Board of the National Academies, Washington DC, December 2010.

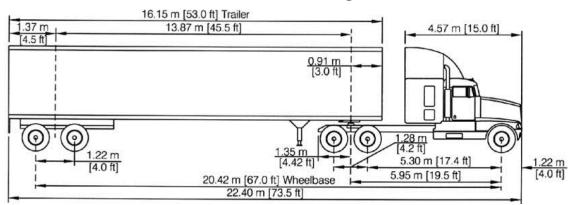
during both peak hours, which indicates satisfactory levels of delay. More detail on the analyses can be found in Appendix D.

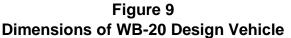
#### 7 IMPROVEMENT ALTERNATIVES

MPO staff developed and analyzed three alternatives to address issues at the study intersection. Because the primary issue identified by the study was the inability of trucks to perform turning movements safely in the intersection, MPO staff designed each alternative with a modified intersection geometry that would accommodate all truck turning movements.

#### Design Method

The required intersection dimensions and curb radii were calculated based on AutoTURN truck swept path simulations conducted by MassDOT for the purposes of this study.<sup>10</sup> An interstate semitrailer WB-20 (also known as WB-65 or WB-67) was used as the design vehicle in those simulations (Figure 9).





MPO staff did not have direct access to the AutoTURN software so the proposals were based on a set of potential truck paths. Using an iterative approach where the simulation is run multiple times as different geometries are tested would yield more precise (and also more conservative, that is, less land taking) designs. This process can be undertaken later in the design phase.

#### Space Requirements

As shown by the property lines in Figure 10, the right-of-way at the intersection extends only to the back of the sidewalk. Almost any effort to accommodate larger turning radii will therefore require either removing the sidewalk or

<sup>&</sup>lt;sup>10</sup> Transoft Solutions, AutoTURN Swept Path Analysis Software, Version 10.2. Richmond, British Columbia.

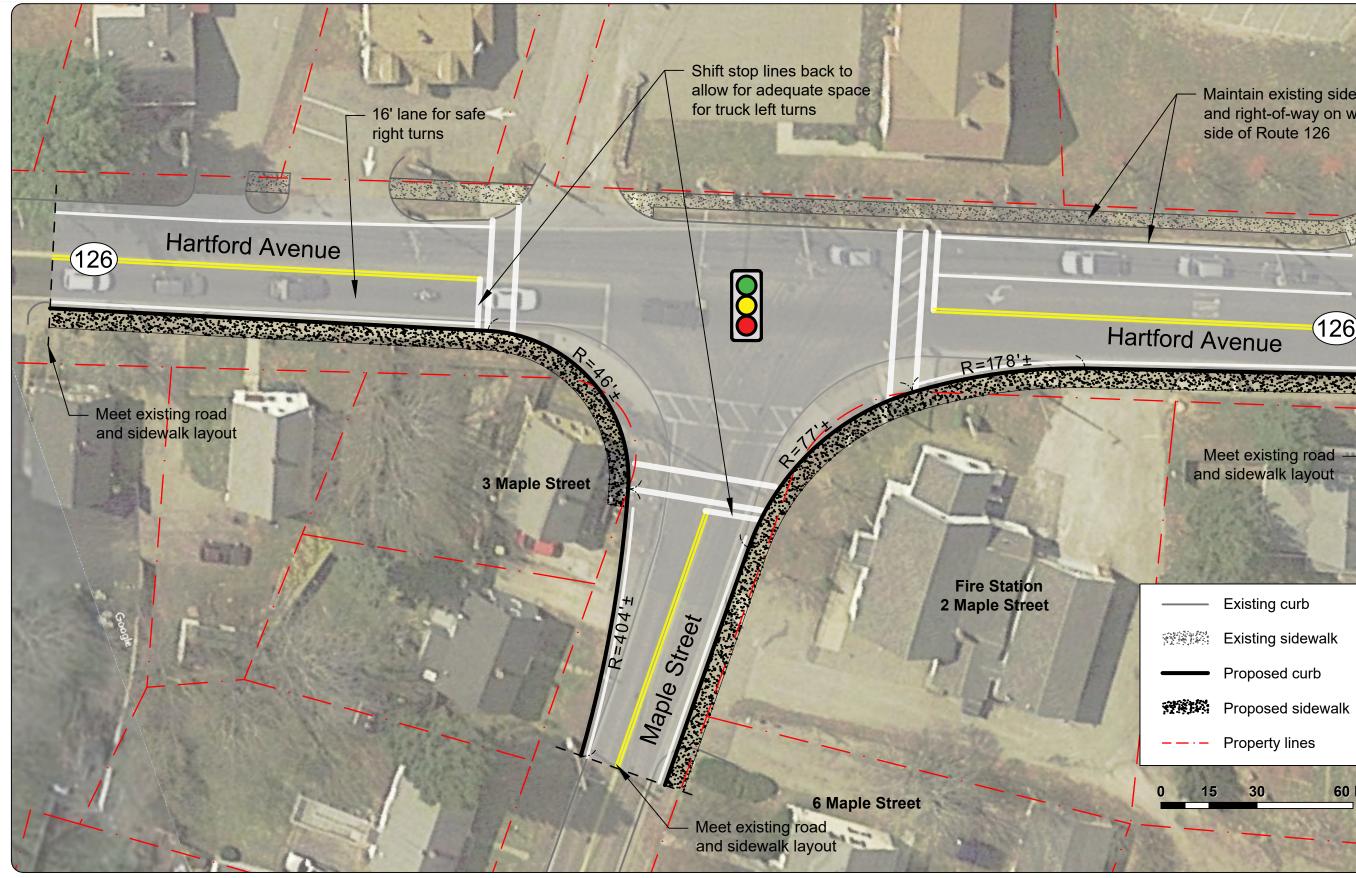
expanding the right-of-way through land takings. Fortunately, the property at the north corner of Maple Street (2 Maple Street) is used as an auxiliary garage for the fire department and is already owned by the Town. The structures are more than 20 feet from the back of sidewalk, allowing a portion of this property to be used for expansion of the right-of-way. The property at the south corner of Maple Street (3 Maple Street) contains a private residence that is built very close to the existing back of sidewalk. However, the Town believes the property could be acquired and has expressed willingness to include this acquisition in design proposals.

#### 7.1 Alternative 1: Increase Curb Radius, Shift Stop Lines Back

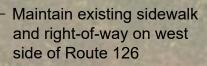
Alternative 1, shown in Figure 10, accommodates truck-turning movements with a combination of wider curb radii and shifted stop lines. Route 126 and Maple Street are maintained on their original alignments.

Figure 11 and Figure 12 show how Alternative 1 accommodates the turning movements of a WB-20 semitrailer. Because the rear wheels of the truck lie so far behind the pivot point at the back of the cab (45.5 feet for a WB-20), the end of the trailer will tend to cut across the inside of any turn these trucks make. During right turns the rear wheels may ride up on the sidewalk and endanger pedestrians if the curb radius is too tight. In Alternative 1 the curb radius on the northern corner of Maple Street is increased to about 77 feet to accommodate trucks turning right from Maple Street onto Route 126 northbound, and the curb radius on the southern corner of Maple Street is increased to about 46 feet to accommodate trucks turning right from 126 northbound onto Maple Street. In both cases the curb would be moved back up to eight feet, and because the existing right-of-way extends only to the back of the sidewalk, roughly 830 ft<sup>2</sup> of land takings would be required to maintain the six-foot sidewalk present in these locations.

On the other hand, during left turns, the rear wheels of a truck drift into the middle of the roadway. This may bring them into conflict with vehicles waiting at the opposite approach and can endanger those drivers. To address this, the stop lines on the westbound and northbound approaches in Alternative 1 are moved away from the intersection to give the rear wheels on the truck enough space to return to the lane of travel. The stop line on Route 126 northbound is moved back about 35 feet to bring it out of the path of trucks turning left from Maple Street onto Route 126 southbound, and the stop line on Maple Street westbound is moved back about 25 feet to bring it out of the path of trucks turning left from Route 126 southbound onto Maple Street. Because there are no left turns onto Route 126 northbound, the stop line on the southbound approach is not in conflict and is kept in its original position.







126

# Hartford Avenue

Meet existing road —/ and sidewalk layout

Existing curb Existing sidewalk Proposed curb Proposed sidewalk Property lines \_ . \_ . 60 FT 15 30

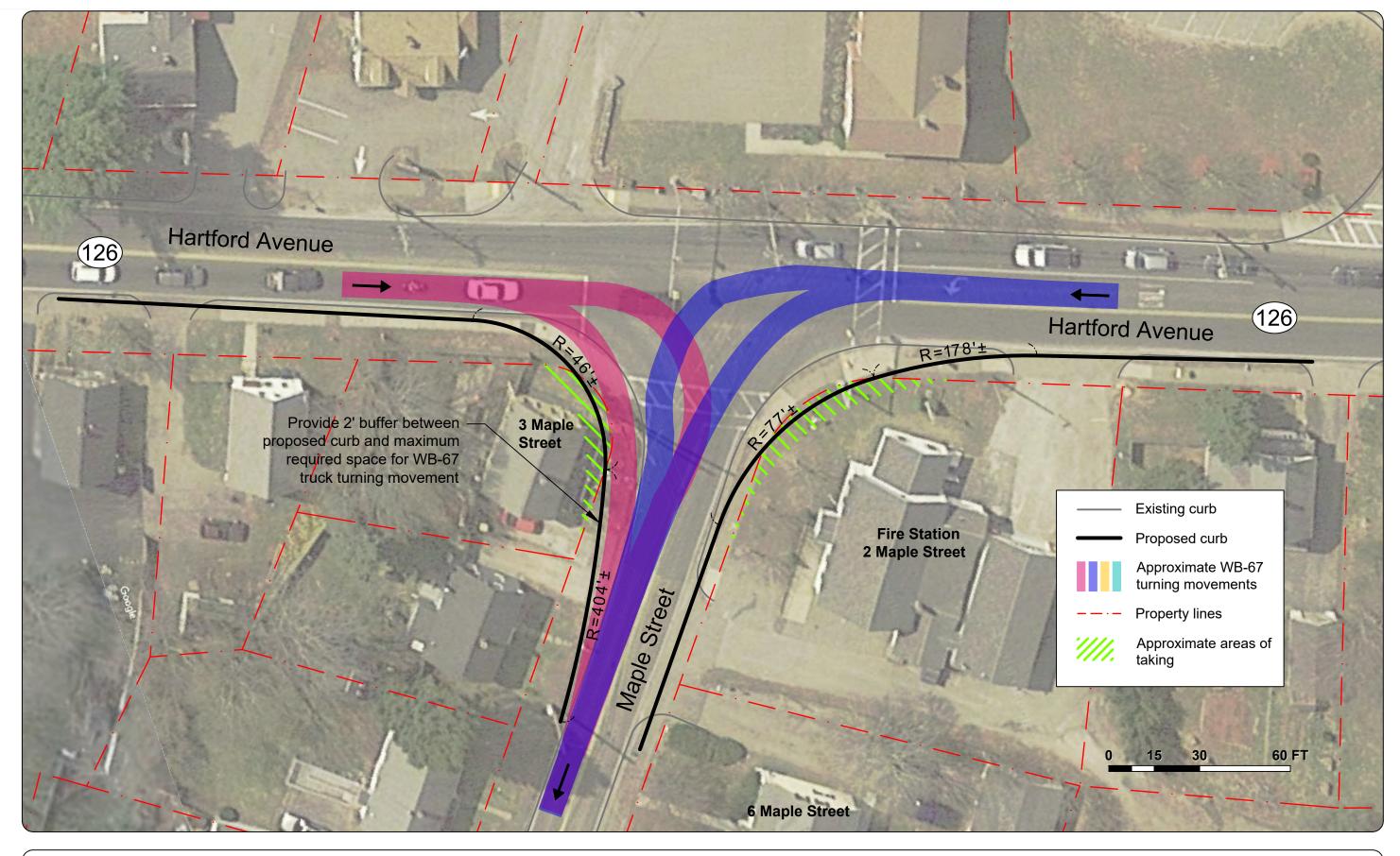




Figure 11 Alternative 1: Truck Paths Turning Onto Maple Street Redesign of Hartford Avenue and Maple Street Intersection

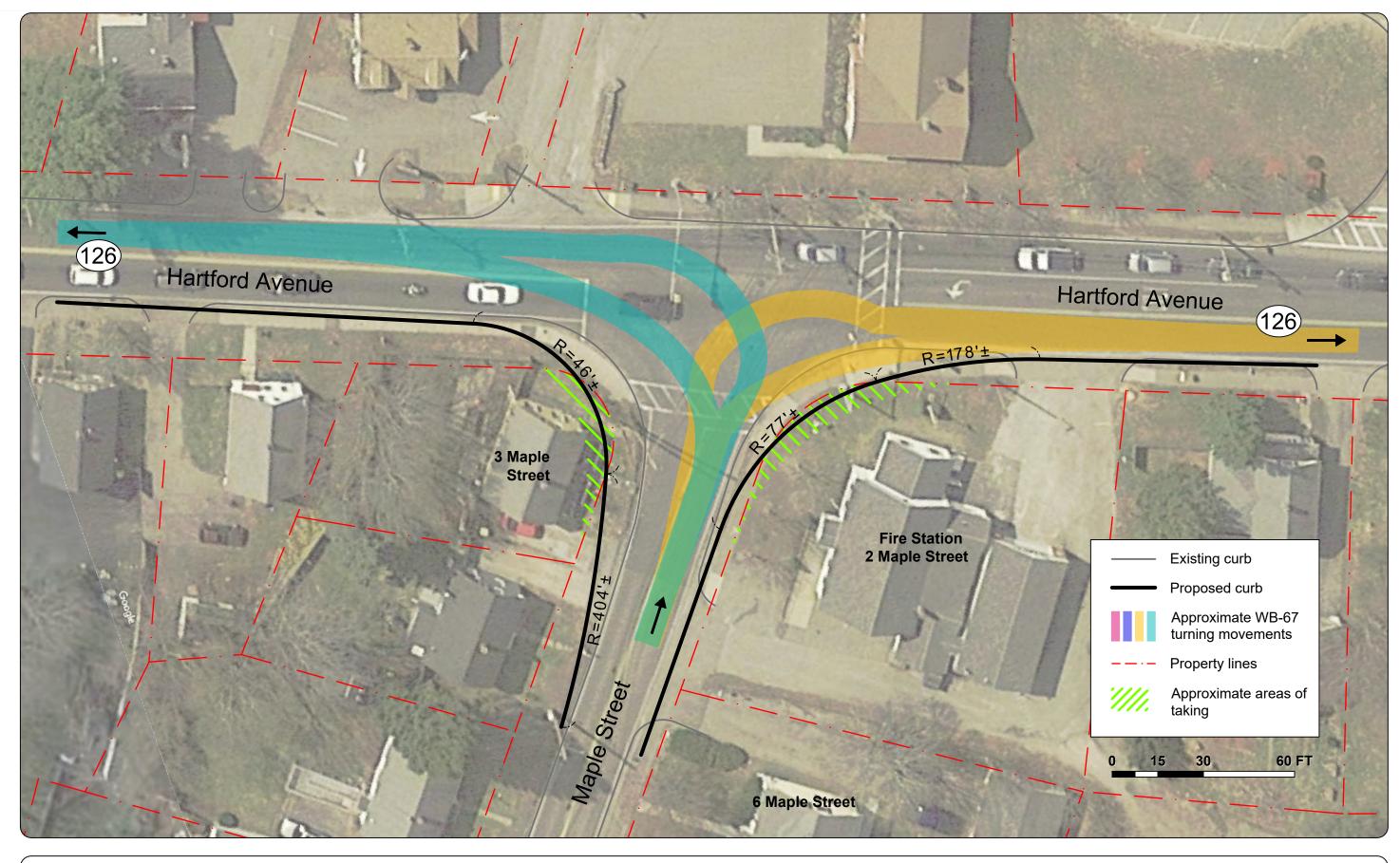




Figure 12 Alternative 1: Truck Paths Turning From Maple Street Redesign of Hartford Avenue and Maple Street Intersection

In addition to the geometric modifications already mentioned, Alternative 1 would include the following improvements:

- A third crosswalk added on the northbound approach crossing Route 126
- Pushbutton pedestrian signals
- Updated signal equipment and clearance times to meet MassDOT standards
- Emergency preemption system and better vehicle detection system
- Additional signal heads for improved visibility on the Maple Street approach

MPO staff estimate the cost of Alternative 1 at between \$1 million and \$1.5 million. This includes design and construction but excludes potential right-of-way acquisition.

# 7.2 Alternative 2: Shift Alignments on Route 126 and Signalize Driveway on the West Side of the Intersection

In Alternative 1, the stop lines must be shifted back a significant distance. This has the effect of making the intersection much larger and forces pedestrians to travel further out of their way to reach the crosswalk. Alternative 2, shown in Figure 13, aims to keep the stop lines closer to their original positions. To accomplish this, the Maple Street approach is widened slightly and the Route 126 alignment is shifted eight to 10 feet west. This helps create a more perpendicular approach from Maple Street to avoid forcing vehicles to make a difficult oblique angle turn as they are currently required to do. The degree to which Maple Street can be realigned is limited by the residential property to the east of the garage (6 Maple Street), which is only 100 feet from the intersection and extends to the back of the sidewalk. The stop line on the northbound approach must still be shifted back slightly for left turns from Maple Street and to accommodate the crosswalk. The trajectories of eastbound and westbound truck traffic can be seen in Figure 14 and Figure 15.

Another feature of Alternative 2 is that the driveway on the eastbound leg of the intersection is shifted 40 to 60 feet north and added to the intersection as a signalized approach. This helps to improve safety and reduce the confusion that results from having an unsignalized driveway entering almost directly into the intersection. Aligning this approach also provides some extra maneuvering room for turning heavy vehicles.

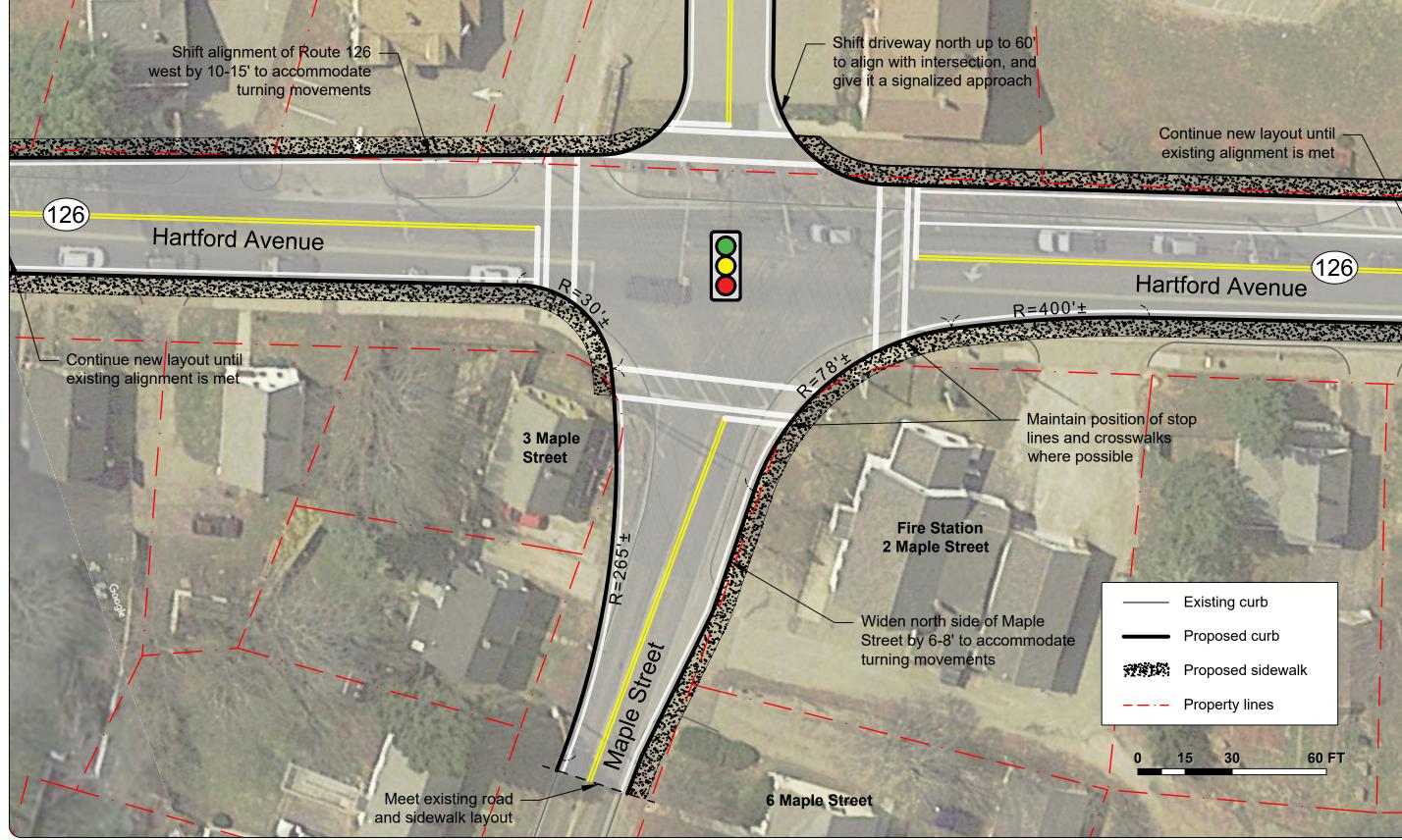
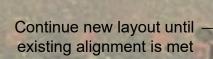




Figure 13 **Alternative 2: Proposed Geometry Redesign of Hartford Avenue and Maple Street Intersection** 



### (126) Hartford Avenue

10

Maintain position of stop lines and crosswalks

> Existing curb Proposed curb 979)Q Proposed sidewalk Property lines 15 30 60 FT

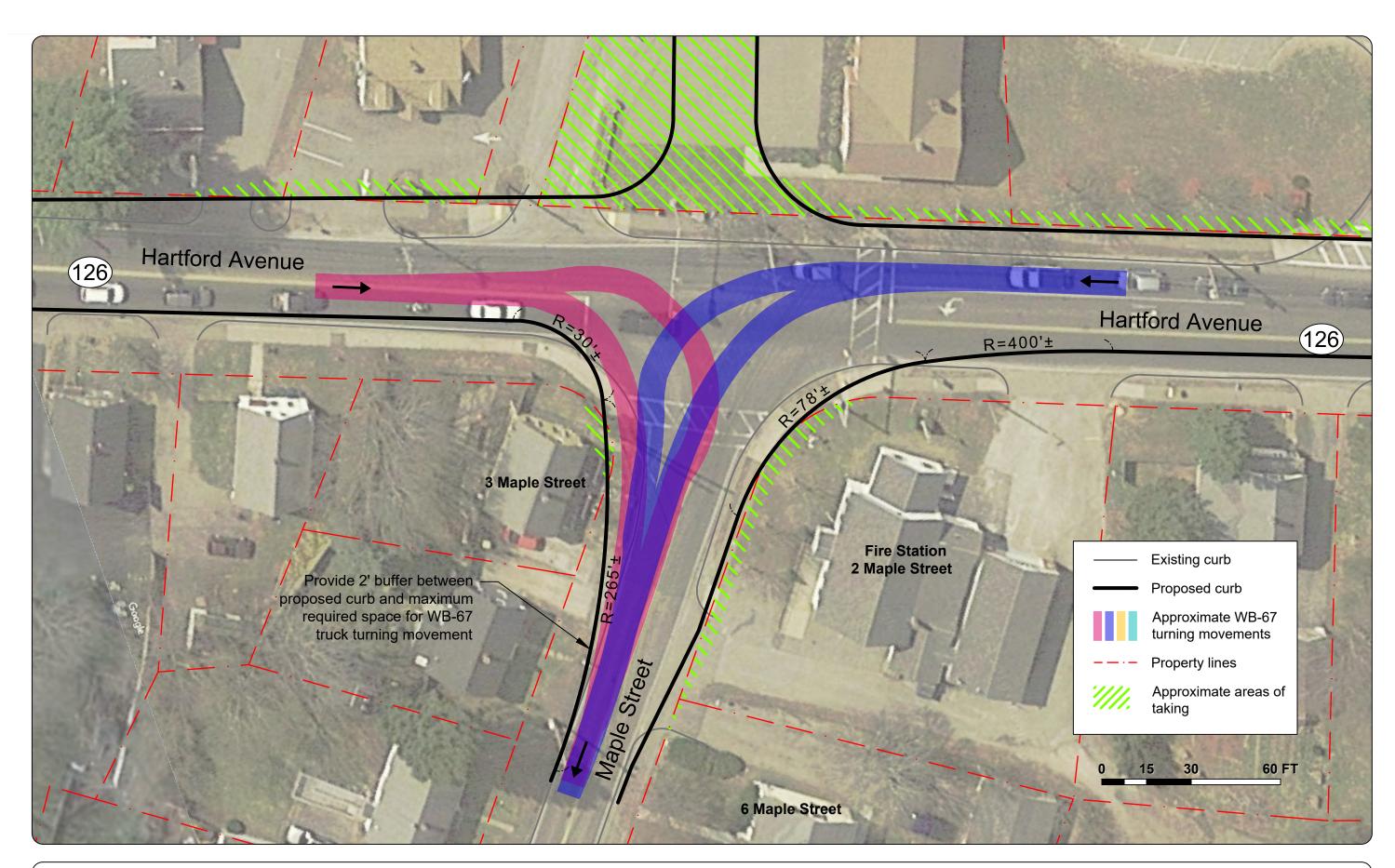




Figure 14 Alternative 2: Truck Paths Turning Onto Maple Street Redesign of Hartford Avenue and Maple Street Intersection

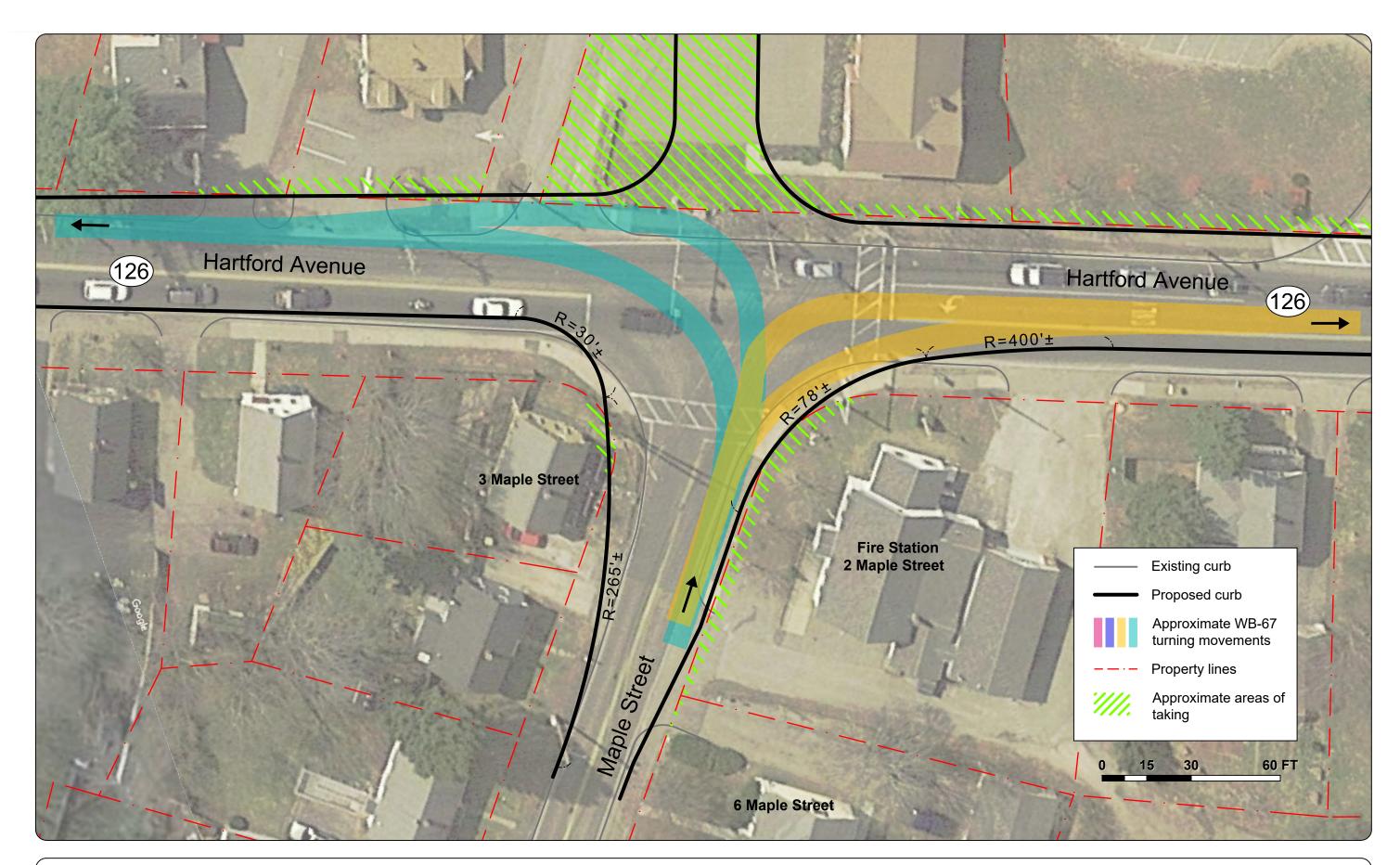




Figure 15 Alternative 2: Truck Paths Turning From Maple Street Redesign of Hartford Avenue and Maple Street Intersection

Alternative 2 requires more right-of-way acquisition than Alternative 1. However, because the building corner of 3 Maple Street is so close to the existing right-of-way, it would be expected that the property would have to be acquired as part of either alternative. If that is the case then it would make sense to use as much of that property as necessary.

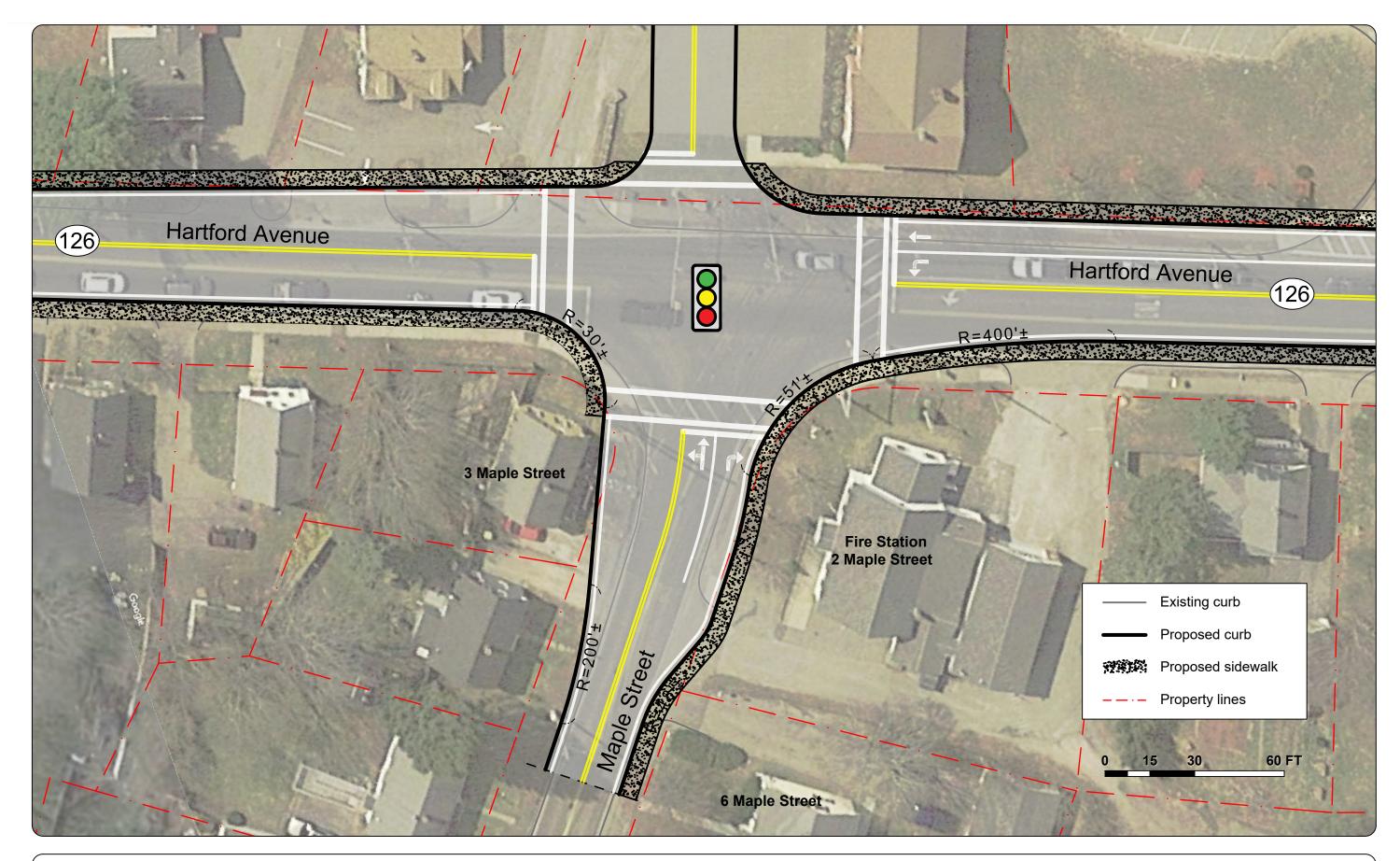
As drawn, Alternative 2 also calls for some takings on the west side of Route 126, both to shift the alignment of Hartford Avenue west and to shift the eastbound driveway north. The Town believed that takings from this property (324 Hartford Avenue) might also be acquired at a reasonable price and wanted to consider it among the design options at the intersection. The amount of takings, if any, on that side of the intersection could be decided later during the design process.

Alternative 2 also includes the same pedestrian improvements and signal standardizations discussed in Alternative 1. MPO staff estimate the design and construction cost of Alternative 2 to be between \$1.5 million and \$2 million.

#### 7.3 Alternative 3: Add Turn Bay on Maple Street Approach

Alternative 3, shown in Figure 16, incorporates some of the suggestions from the Town of Bellingham into the original Alternative 2 design. Although the 2025 level of service is acceptable, the anticipated commercial and industrial growth on Maple Street has the potential to sustain traffic growth well beyond that threshold. Alternative 3 adds a right-turn bay from the Maple Street approach to increase the overall capacity of the intersection with the goal of extending the functional life of the design proposal. Like Alternative 2, this design also assumes that properties adjacent to Maple Street will have to be purchased and seeks to fully use this extra real estate. The estimated cost for Alternative 3 would be similar to the \$1.5 million to \$2 million range of Alternative 2.

Because it was added at a later stage in the project, MPO staff did not have the opportunity to test Alternative 3 with AutoTURN simulations. The geometry in Figure 16 gives a conceptual design only.





#### 7.4 Level of Service of Proposed Alternatives

Table 4 summarizes the level of service analysis for the proposed alternatives. The 2025 future conditions project a uniform 5 percent traffic increase in the study area. More detail on the analyses can be found in Appendix D.

Alternative/	Move-	AM	AM	АМ	РМ	PM	РМ
Approach	ment	LOS	Delay <sup>a</sup>	Queue	LOS	Delay	Queue
2025 No-Build	-	_	_	-	_	_	-
Route 126 Northbound	LTR	D	38.4	#642	С	34.5	#653
Route 126 Southbound	L	А	9.5	29	С	21.3	123
Route 126 Southbound	TR	А	9.4	176	А	8.4	259
Maple Street	LT	Е	55.4	#347	D	49.5	#205
Maple Street	R	Е	55.4	#347	D	49.5	#205
Intersection Average	All	С	32.4	-	С	24.5	_
Alternative 1	-	_	-	-	_	_	-
Route 126 Northbound	LTR	D	40.7	#648	D	35.5	#659
Route 126 Southbound	L	Α	9.5	29	С	20.1	118
Route 126 Southbound	TR	Α	9.4	176	А	8.4	259
Maple Street	LT	Е	55.4	#347	D	49.9	#205
Maple Street	R	Е	55.4	#347	D	49.9	#205
Intersection Average	All	С	33.4	-	С	24.8	-
Alternative 2	-	-	-	-	-	-	-
Route 126 Northbound	LTR	D	53.9	#719	С	33.2	#626
Route 126 Southbound	L	А	9.1	34	А	8.1	59
Route 126 Southbound	TR	В	11.4	210	А	8.8	261
Maple Street	LT	Е	56.7	#358	E	56.4	#217
Maple Street	R	Е	56.7	#358	E	56.4	#217
Driveway	LTR	С	23.0	10	С	30.2	21
Intersection Average	All	D	40.4	-	С	23.3	-
Alternative 3	-	-	-	-	-	-	-
Route 126 Northbound	LTR	С	26.3	511	В	20.0	433
Route 126 Southbound	L	А	5.2	23	А	6.3	49
Route 126 Southbound	TR	А	6.8	144	А	6.2	219
Maple Street	LT	D	42.7	#159	D	40.3	99
Maple Street	R	D	47.2	#222	D	39.6	108
Driveway	LTR	С	30.4	12	С	32.1	22
Intersection Average	All	С	23.8	-	В	15.2	-

Table 4Levels of Service of Proposed Alternatives

L = left turn. LOS = levels of service. R = right turn. T = straight through.

<sup>a</sup> Delay in seconds per vehicle.

<sup>b</sup> 95th percentile queue length in feet.

Notes:

# = the 95th percentile volume exceeds capacity.

Source: Central Transportation Planning Staff.

The proposed modifications are mostly geometric; only adding a turn lane in Alternative 3 and signalizing the driveway in Alternatives 2 and 3 significantly influenced level of service. Shifting stop lines back in Alternative 1 required an extra one-half second of all-red time on the northbound approach to clear the intersection, but this change had negligible effects on capacity.

The unsignalized intersection at the Stall Brook School driveway was also included in the analysis, although it did not have operational issues under any scenario. Those results can also be found in Appendix D.

#### 8 CONCLUSIONS AND NEXT STEPS

#### 8.1 Conclusions

The above analyses and evaluation supports the need for renovations that would improve access for heavy vehicles and enhance safety for pedestrians and motorists.

All of the proposed alternatives address the issue of truck maneuvers. The scale of the changes varies, however, with Alternative 1 providing the lowest cost option and Alternative 3 providing the most comprehensive solution.

- Alternative 1 minimizes construction and takings by only proposing modifications to the east side of Route 126
- Alternative 2 improves safety and pedestrian comfort by keeping the intersection geometry tighter
- Alternative 3 adds a turn bay to preemptively address operational issues that might be caused by future traffic growth

Deciding between the three alternatives hinges upon:

- Available budget and funding sources
- Ability to acquire the property at 3 Maple Street and/or 334 Hartford Avenue
- Refined projections for growth and land use in the region
- On-ground survey to determine the feasibility of each option
- Feedback from stakeholders

If found to be suitable after considering the factors listed above, MPO staff recommends Alternative 3 because it represents the most complete solution to the issues encountered at the study intersection.

#### 8.2 Next Steps

The Town of Bellingham has jurisdiction of the intersection and is responsible for renovations to improve safety, mobility, connectivity, and operations. This study gives the city an opportunity to review the needs of the intersection and plan for design and engineering. The next step would be to select the preferred alternative that is sensitive to the goals and needs of stakeholders, and then advance the project through the planning process. These steps will depend upon cooperation between MassDOT, the Town of Bellingham, and the MPO to begin the project notification and review process, and complete the project initiation form. After completing the initial steps, the Town of Bellingham and MassDOT can start preliminary design and engineering to place the project in the Transportation Improvement Program. Transportation decision making is complex, and influenced by factors such as financial limitations and agency programmatic commitments. Project development is the process that takes transportation improvements from concept to construction (see Appendix E for an overview of this process).

This study supports the MPO's visions and goals, which include increasing transportation safety, maintaining the transportation system, advancing mobility and access, reducing congestion, and expanding the opportunities for walking and bicycling, while also making them safer. If implemented, the improvements proposed in this report would increase traffic safety and modernize the roadway to accommodate all users.

cc: Erin Kinahan, MassDOT District 3, Joseph Frawley, MassDOT District 3 Appendix A: Comments and Selection Process Hartford Avenue (Route 126) and Maple Street Intersection Redesign Study Lower Level Meeting Room Bellingham Municipal Center, 10 Mechanic Street May 4, 2018

#### **Attendance**

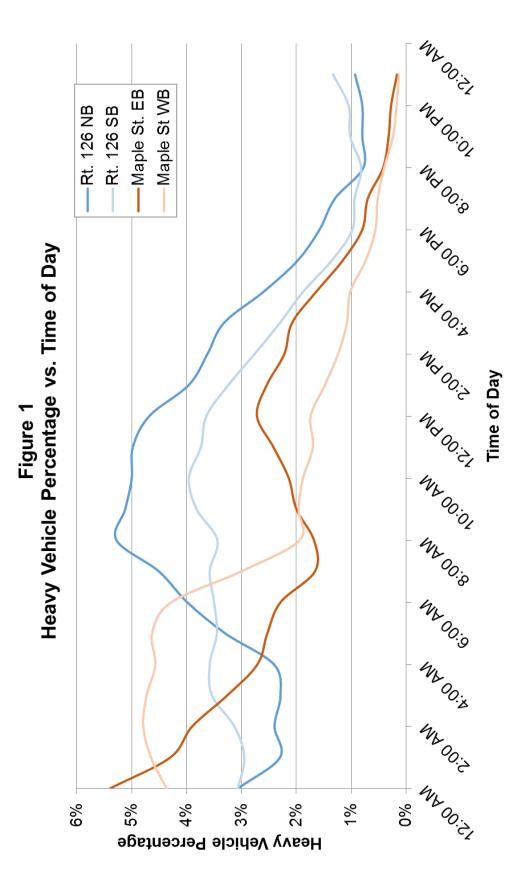
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#### Summary and Updates from Meeting with Town of Bellingham

- CTPS presented their two proposed alternatives to address truck and pedestrian accommodations at the intersection of Route 126 and Maple Street. A PDF copy of the presentation has been attached to this email.
  - The proposed alternatives were well received, with preference expressed towards Alternative 2 (geometric changes on both Hartford Avenue and Maple Street, align driveway with intersection) to better meet expected future growth.
  - Additionally, some interest was shown in adding a turn bay for trucks leaving Maple Street, either as a component of one of the alternatives or as a third option. CTPS will investigate this possibility and include its findings in the final memo.
- Interest was expressed in permitting Right Turn on Red for traffic leaving Maple Street. However, according to CTPS analyses the sight distance at this intersection is small enough that right turn on red should probably be prohibited under existing conditions. Making right-turn-onred a priority may require removal of the fence or city garage building on the north side of the intersection.
- The telephone pole nearest the curb on the southern corner of Maple and Hartford has been removed over the past week or two. The associated cables are now carried by the second pole closer to the house at #3 Maple Street.
- Some of the attendees were curious to know when specifically the highest truck flows occurred through the study intersection. A figure showing heavy vehicle percentage vs. time can be found on the next page of these minutes.
- Attendees also discussed how this project would be funded such as through the MPO TIP and MassWorks.

#### **Follow-up Tasks**

- Town of Bellingham: Review the included presentation and provide feedback to CTPS by Tuesday, May 15.
- CTPS: Incorporate feedback from the Town of Bellingham, and prepare final memo to be published in July/August.



Hartford Avenue and Maple Street Intersection Redesign Study Lower Level Meeting Room Bellingham Municipal Center, 10 Mechanic Street May 4, 2018

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Hartford Avenue (Route 126) and Maple Street Intersection Redesign Study Lower Level Meeting Room Bellingham Municipal Center, 10 Mechanic Street February 22, 2018

#### **Attendance**

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#### Summary of concerns and ideas discussed at meeting

- The major issue is truck maneuvers to/from Maple Street. Heavy vehicles have a difficult time turning onto and off of Maple Street from Hartford Avenue (Route 126).
  - Taking a right from Hartford Avenue northbound onto Maple Street is the shortest route to access Maple Street from I-495, although the turn onto Maple Street is difficult for trucks because of a substandard curb radius. This fact is demonstrated by the telephone pole in front of the property on the southeast corner of the intersection, which is continually being hit by turning trucks.
  - The left turn out of Maple Street is also difficult heavy vehicles leaving Maple Street properties are technically required to head west to Route 140 instead of using the Hartford Avenue and Maple Street intersection, which is an inconvenience.
  - Most of the complaints from residents about this intersection relate to turning heavy vehicles.
- Future development: Maple Street already sees a high volume of truck traffic, and is primed for growth in the near future.
  - All the land surrounding Maple Street between Pine Street and Route 140 is zoned industrial. Much of this is either undeveloped (woodland or swamp), or underdeveloped properties where new businesses have expressed interest.
  - Existing industrial businesses include several mulch distributors, a power station, and a construction company.
  - Future industrial properties include a new 450,000 SF industrial warehouse going in across the street from Camp Bow Wow, as well as a second warehouse.
  - Additionally, the Maplegate Country Club which is mostly located in Franklin recently changed ownership and may be up for sale, freeing up a huge amount of real estate for industrial development.
  - This area is in high demand because it is one of the few suitable sites for warehouses right off I-495. Additionally, its location makes it a good stopover location for redistributing loads (e.g. mulch from Canada) to match weight regulations in Connecticut and Rhode Island.
- The Hartford Avenue and Maple Street intersection is the limiting factor for further development along Maple Street. The Town of Bellingham has previously made several investments in Maple Street.
  - A redesign of the intersection at the other end of Maple Street (with Route 140) is already planned. The consultant for this project is BETA Group and the work will be paid for by a MassWorks grant along with funds from a private developer.

- Bellingham has spent 1.0 million on Maple Street itself to improve drainage, widen slightly, and repair wear and tear from the heavy vehicle traffic. Kleinfelder was the design contractor for this work.
- Potential solutions and other ideas discussed at the meeting
  - The Town of Bellingham is potentially open to options involving land takings necessary for a satisfactory solution.
  - Moving the Maple Street stop line back would present a low-cost solution, but the Town of Bellingham expressed interest in pursuing more long-term solutions.
  - The driveway across Hartford Avenue from Maple Street is private property but is sometimes used for school traffic. Shifting this driveway north and making the intersection 4-way could be an option. The property to the north of the driveway is privately owned but may be up for sale.
  - The property on the northeast corner of the intersection is a garage used by the fire department (storage only) and is publicly owned, so could be used for right-of-way if necessary.
  - The house on the southeast corner was up for sale a few times. The right-of-way is only 10 or 15 feet from the building there. It has been up for sale a few times, for around \$175,000.
- Follow-up tasks
  - Town of Bellingham: send recent and future developments and signal timing plans to CTPS



Stephanie Pollack, MassDOT Secretary and CEO and MPO Chair Karl H. Quackenbush, Executive Director, MPO Staff

#### TECHNICAL MEMORANDUM

- DATE: January 18, 2018
- TO: Boston Region MPO
- FROM: Seth Asante, Chen-Yuan Wang, and Ben Erban
- RE: Safety and Operations Analyses at Selected Intersections: Federal Fiscal Year 2018

#### 1 BACKGROUND

This memorandum presents the results of Task 1 (Select Study Locations) of the work program for Safety and Operations Analyses at Selected Intersections: Federal Fiscal Year (FFY) 2018.<sup>1</sup>

This study builds on recommendations generated by the Boston Region Metropolitan Planning Organization's (MPO) Congestion Management Process (CMP) to address safety and congestion problems at intersections in the MPO area. Several similar studies were completed in previous funding years and received favorable responses from municipalities, which included appreciation of the MPO's assistance with the conceptual design of low-cost improvements and the planning and implementation processes.

Previous studies examined large, complex intersections, simpler intersections, and locations that include two or more adjacent intersections. The focus for FFY 2018 is on simpler intersections. Locations that would potentially require major geometry redesigns, such as grade separation or adding travel lanes on an arterial roadway, were considered to be less suitable for this study.

As in the past, the basic requirement for a location to qualify as a study candidate is that it must be located on an arterial roadway in the Boston Region MPO where 1) it has safety and operational concerns and 2) the agencies and/or municipalities with jurisdiction over the roadway are committed to implementing recommended improvements.

<sup>&</sup>lt;sup>1</sup> Karl H. Quackenbush, CTPS Executive Director, memorandum of a work program to the Boston Region MPO, "Work Program for Safety and Operations Analyses at Selected Intersections," November 16, 2017.

#### 2 SELECTION PROCEDURE

The study selection process consisted of the following four steps completed by the MPO:

- 1) Generate a list of potential intersection study locations then narrow it to 10 locations
- 2) Gather detailed data for each of the 10 locations
- 3) Apply specific criteria to examine potential study locations more closely
- 4) Score and rate the 10 locations, and assign low, medium, or high priority to each intersection location

#### 2.1 Generating List of Potential Locations

MPO staff used the following sources to develop an initial list of nearly 50 potential study locations in the MPO area:

- FFY 2016 safety and operations list of potential candidates
- Suggested locations from Unified Planning Work Program outreach

The following exclusion criteria were developed to narrow the list of locations:

- Located in a municipality that has been selected for this study within the past three years
- Located in a subregion that has been well- or over-represented in past subregional priority corridor projects in terms of the proportion of population or Massachusetts Department of Transportation (MassDOT) top-200 high-crash locations in the region
- Studied by MPO staff or another agency; included in a Transportation Improvement Program (TIP) project with a status of "advertised" or "programmed," or included in an active MassDOT or other agency project that is in design (at 25 percent or higher design status), in construction, or recently completed
- Considered part of a larger potential study area, such as a highway interchange or a long traffic corridor with an extensive area of congestion
- Considered not at-grade

#### 2.2 Gathering Detailed Data

Staff gathered data to support the exclusion criteria and eliminated locations that were not suitable. The assembled data for 10 intersection locations in 10 municipalities in the MPO region are listed below.

 MassDOT's 2015 Road Inventory File. To collect the following information for each major arterial segment in each intersection location: roadway jurisdiction, National Highway System (NHS) status, and annual average daily traffic (AADT)

- MassDOT's Transportation Data Management System. Recently updated AADT counts were retrieved from MassDOT's online database
- MassDOT's 2010–14 Crash Database. Identify high-crash locations and numbers of crashes
- MPO CMP Data on Arterial Congestion. Determine travel-time index (that is, travel time in the peak period divided by travel time in free-flow conditions) for each major arterial segment intersection location
- MPO Data on Bike Network Gaps and MassDOT Bike Facilities. Identify bicycle needs—including connectivity—and accommodation
- Data on Massachusetts Bay Transportation Authority (MBTA) Bus Service Performance and Passenger Load. Determine the percentage of bus trips that do not adhere to the schedule (late service) or to passenger load standards (crowding)
- Data on MBTA Subway and Commuter Rail Lines. Identify locations serving MBTA stations
- Data from the following sources were also included:
  - Data selected from MassDOT's project-information and roadway safety audit databases
  - The MPO's 2016-20 TIP projects
  - o MPO planning (and other) studies
  - Municipal websites (to obtain data on projects, studies, and TIP projects planned or programmed for each arterial segment)

Table 1 (at the end of this memorandum) presents the data assembled for each intersection location, community, Metropolitan Area Planning Council (MAPC) subregion, MassDOT district office, jurisdiction, equivalent property damage only crashes, total crashes, fatal crashes, injury crashes, property damage only and non-reported crashes, bicycle and pedestrian crashes, top-200 crash clusters, crash clusters that are eligible for Highway Safety Improvement Program (HSIP) funding, transit routes, a list of relevant studies or projects, and staff comments. The table also shows the results of applying the selection criteria and the priority rating, which was performed in the fourth step of this process (described below).

#### 2.3 Applying Criteria

MPO staff further examined the intersection locations by applying the five criteria cited below (each item is worth one point):

- Safety Conditions, 0–2 Points
  - Location has an estimated crash rate that is higher than the district average
  - Location has a significant number of pedestrian and bicycle crashes per year (more than three), or has truck traffic safety concerns

- Multimodal Significance, 0–2 Points
  - o Location needs improved transit, bicycle, or pedestrian facilities
  - Location has a high volume of truck traffic serving regional commerce
- Regional Significance, 0–2 Points
  - Location carries a significant portion of regional traffic (AADT is greater than 15,000 on at least one intersecting road)
  - Location is essential for the region's economic, cultural, or recreational development
- Regional equity, 0-2 Points
  - Location is in an MPO subregion that is at least slightly underrepresented in previous safety and operations analyses in terms of the proportion of population or number of MassDOT top-200 highcrash locations in the region
  - Location is in an MPO subregion that is very under-represented in previous safety and operations analyses in terms of the proportion of population or number of MassDOT top-200 high-crash locations in the region
- Implementation Potential, 0–2 Points
  - Location has strong potential for implementation based on the urgent need for safety improvements
  - Location is proposed or endorsed by its roadway administrative agency or agencies and has strong support from other stakeholders (for example, municipalities, MassDOT, and subregions)

In addition, no two locations in the same town would be selected.

#### 2.4 Scoring and Rating

Intersection locations with a score of four or fewer points were rated low priority; those with a score of five to seven points were rated medium priority; and those with a score of eight or more points were rated high priority. Five locations were given a high-priority rating and four a medium-priority rating by MPO staff based on safety, operations, multimodal and regional significance, and support from agencies and municipalities.

Staff examined the high-priority segments more closely. Locations within the following parameters were not suitable candidates for this cycle of safety and operations analyses:

- Locations that were recently or are currently under study
- Locations that exhibited a density of closely spaced intersections that suggest that a corridor study is needed
- Locations that were selected for the FFY 2018 Subregional Priority Corridors study

#### 3 SELECTED INTERSECTIONS FOR STUDY

Based on the evaluation above, staff selected two intersections for study: 1) Route 1A (Main Street) at Cherry Street, Monument Street, and Arbor Street in Wenham; and 2) Route 126 (Hartford Avenue) at Maple Street in Bellingham.

 Route 1A (Main Street) at Cherry Street, Monument Street, and Arbor Street in Wenham: The Town of Wenham and MassDOT District 4 requested MPO staff to study three major intersections on Route 1A from Cherry Street to Arbor Street. The primary issues raised were safety and operational concerns for users of all modes, including pedestrians and bicyclists.

The three intersections are located close to each other within a short distance of 750 feet and serve a high volume of traffic on the regional arterial of Route 1A corridor. Additionally, several properties are located adjacent to these intersections, including the town hall, police department, fire department, the Maples Retirement Home, and First Church. The combination of these factors has caused safety concerns for all the users, especially for residents frequently visiting the area.

All three intersections are currently unsignalized, and preliminary traffic signal needs analyses performed by MassDOT show that they satisfy the first three warrants of Manual on Uniform Traffic Control Devices. However, the three intersections should be further examined together in a comprehensive study under the existing town center context.

 Route 126 (Hartford Avenue) at Maple Street in Bellingham: The Town of Bellingham requested MPO's assistance in addressing the safety and operational concerns at this intersection, especially on the truck operational and safety issues.

The Town expressed that the intersection at Hartford Avenue and Maple Street carries a high proportion of truck traffic and is undersized to accommodate large commercial vehicles safely and efficiently. The intersection is just one-half mile south of the interchange of Interstate 495 and Route 126, where a number of large commercial uses exist. Meanwhile, a significant portion of Maple Street, currently zoned industrial, houses a power plant, multiple warehouses, mulch- and lumber-producing facilities, and vacant land for future developments.

In addition, an elementary school that serves all of North Bellingham is located on Route 126, less than 100 feet north of the intersection. The traffic and pedestrian access to the school should also be considered in further study. The intersection is suitable for this study because of the issues and concerns from these different travel modes.

Staff also evaluated the pedestrian accommodation and safety improvement needs for the two locations by applying the Pedestrian Report Card Assessment that the MPO recently developed.<sup>2</sup> The two selected locations are highly qualified for pedestrian accommodation or safety improvement requirements. Appendix A contains detailed results of the assessments.

#### 4 SUMMARY

The recommended intersection locations meet the selection criteria of this study because of their potential for safety and operations improvements. The work scope for this study assumed that "as many as three" locations would be selected. Staff selected two locations that contain a total of four intersections. Appendix B contains the support letters from MassDOT and stakeholders in Wenham and Bellingham.

Staff will submit these recommendations to the MPO for discussion. If the MPO endorses the study selections, staff will meet with officials from Wenham, Bellingham, and MassDOT to discuss study specifics, conduct field visits, collect data, and perform analyses.

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<sup>&</sup>lt;sup>2</sup> Pedestrian Level-of-Service Memorandum, Ryan Hicks and Casey-Marie Claude, Boston Region Metropolitan Organization, January 19, 2017.

#### TABLE 1. FFY 2018 Safety and Operations for Selected Intersections Selected locations are highlighted in green

	T	T.			T		I.		T			is are nignlignled	a in groon		T					1			
Location	Community	MAPC Subregion	MassDOT District	Jurisdiction	Street 1	Route 1	Street 2	Study, Project, or TIP Project	EPDO Crashes 2012-14		Injury Crashes 2012-14	Bike/Ped Crashes 2012- 14	Top 200 Crash - Clusters 2012- 14	Crash Clusters	Transit Routes	Safety Conditions	Multimodal s Significance	Regional Significance	Regional Equity	Implementation Potential	Total Score	Rating	Comments
1	Wenham	NSTF	4	MassDOT	Main Street		Cherry Street Monument Street Arbor St / Friend Ct	None	76	36	10	1	0	0	None	2	2	2	2	2	10		Wenham and MassDOT District 4 requested MPO staft to study these three major intersections on Route 1A. The primary issues raised were safety and operational concerns for users of all modes, including pedestrians and bicyclists. To fully address these issues, the three intersections should be examined together under the existing town center context.
2	Bellingham	SWAP	3	Town	Hartford Avenue	Route 126	Maple Street	#604862: Bellingham- Ramp Construction and Relocation, I- 495 At Route 126 (Hartford Avenue) (haff a mile south of location) (TIP project, preliminary design phase, last update 2007) #605239: Bellingham- Franklin- Bridge Preservation - Hartford Ave over I-495 (haff a mile south) (Complete 2012)		8	1	0	0	0	None	1	2	2	2	2	9		The Town of Bellingham requested MPO's assistance in addressing the safety and operational concerns at this intersection, especially on the truck operational and safety issues. A future study should also consider traffic and pedestrian safety from an elementary school adjacent to the intersection.
3	Danvers	NSTF	4	MassDOT	Andover Street	Route 114	Garden Street	Project 605383 Danvers- Peabody- Resurfacing and Related Work on Route 114 (completed in 2011)	97	37	15	1	1	1	None	2	2	2	1	1	8	High	This intersection was studied as part of the FFY 2011 Priority Corridors: Route 114 Study in Danvers. That study proposed improvements for addressing safety and operations at the intersection.
4	Cambridge	ICC	6	DCR and City	Mount Auburn Street an Fresh Pond Parkway		Coolidge Hill Road	None	101	41	15	1	1	1	MBTA 71 and 73	2	2	2	2	0	8		Comments from MPO outreach indicate pedestrian safety issues and traffic congestion and operations concerns at Mount Aubum Street/Coolidge Hill Road. DCR interest is critical for this study due to the proximity of Route 3/Fresh Pond Parkway at Mount Aubum Street.
5	Marlborough	MetroWest	3	MassDOT	Boston Post Road Wes		Northboro Road East (Shopping Plaza)	#601133: Marlborough- Roadway Reconstruction Including Signals, Route 20 (Boston Post Road) From The Northboro C1 To Fetton St. (2004) #608467: Marlborough- Resurfacing And Related Work On Route 20 (Unknown Location) (Planned for 2019 TIP)		68	6	4	0	1	MWRTA Route	2	2	2	1	1	8		A Route 20 study in Marborough is recommended for the MPO FFY 2016 Subregional Priority Corridors Study. This location was not selected because of the geographic equity consideration applied in the selection study locations.
6	Boston	ICC	6	DCR	Jamaicaway		Bynner Street	None	122	50	18	2	1	1	None	1	2	2	1	1	7	Medium	Potential candidate for a safety and operations study. The location is in the current list of Top 200 High-Crash Intersections. The City of Boston expressed interest, but the DCR did not indicate interest.
7	Salem	NSTF	4	Town	North Street	Route 114	Mason Street	#605332: Salem- Bridge Replacement, S-01-001, (St 114) North Street Over North River - Is just south of the intersection. (TIP project, begins 2021) #608521: Salem- Bridge Maintenance, S-01-018 (321), (St 114) North Street Over (St 107) Bridge Street and MBTA - a litele further down (TIP project, begins 2018)	102	45	12	6	1	1	MBTA 465	1	2	2	1	1	7		This location was not selected because the crash cluster at this location includes two signalized intersections and four unsignalized intersections in a half-mile distance. An arterial segment study is more suitable for this location. In active 1A study involving Swampscott. Selem, and Marblehead has been recommended for the MPO FFY 2016 Subregional Priority Corridors Study, and so, because of geographic equity considerations, this location is not recommended for that reason as well.
8	Boston	ICC	6	MassDOT	Columbia Road		Buttonwood Street	#603412: Boston- Traffic Signal And Safety Improvements, Route I-93 Ramps At Columbia Road - is adjacent to intersection. (Complete 2005)	79	27	13	0	0	1	MBTA 8, 18, and 41	2	1	1	2	1	7	Medium	Potential candidate for a safety and operations study. This unsignalized intersection is located between two busy and closely spaced signalized intersections.
9	Newton	ICC	6	City	Commonwealth Avenue	e Route 30	Washington Street	None	22	14	2	1	0	0	MBTA 505	0	2	1	2	1	6	Medium	Potential candidate for a safety and operations analysis.
10	Sherborn	SWAP	3	Town	Washington Street	Route 16	S Main Street (Route 27)	None	46	18	7	0	0	1	None	1	1	1	1	0	4	Low	Location was studied by CTPS and VHB in 2002 and 2004. Improvements were not implemented. A UPWP comment suggested that this could be a good location for demand response signal.

Acronyms and Abbreviations BAT = Brockton Area Transit Authority. CATA = Cape Ann Transit Authority. CTPS = Central Transportation Planning Staff. DCR = Department of Conservation and Recreation. EPDO = Equivalent property damage only. FY = Federal fiscal year. HSIP = Highway Safety Improvement Program. ICC = Inner Core Committee. MAPC = MetroPolitan Area Planning Council. MassDOT = Massachusetts Department of Transportation. MBTA = Massachusetts Bay Transportation Authority. MetroWest = MetroWest Regional Collaborative. MPO = Boston Region Metropolitan Planning Organization. MWRTA = MetroWest Regional Transit Authority. NSPC = North Shore Task Force. SWAP = South West Advisory Planning Council. UPWP = Unified Planning Work Program. MPO = Boston Region Metropolitan Planning Organization. MWR IA = MetroWest Regional Transit Authority. NSPC = North Suburban Planning Council. NST F = North Shore Task Force. SWAP = South West Advisory Planning Committee. TIP = Transportation improvement Pro Selection Criteria Safety Conditions: Intersection has a HSIP-eligible crash cluster, a top-200 high-crash location, and/or a significant number of or HSIP-eligible clusters of pedestrian or bicycle crashes. Congested Conditions: Intersection experiences delays during peak periods. Multimodal Significance: Intersection currently supports transit, bicycle or pedestrian activities, needs improved facilities for these activities, and/or has high truck traffic serving regional commerce. Regional Significance: Intersection is underrepresented in previous safety and operations studies in terms of the proportion of population or number of top-200 high-crash locations. Implementation Potential: Intersection has strong potential for implementation based on the urgent need for safety improvements, is proposed or endorsed by its roadway administrative agency or agencies, and/or has strong support from other stakeholders.

#### Notes

Notes
1. Locations are in order of their ratings based on scoring from selection criteria.
2. EPDO Crash Rating = 10 \* Fatal Crashes + 5 \* Injury Crashes + 1 \* Other Crashes (Property Damage Only or Unknown Severity), based on MassDOT top-200 high-crash locations: 2012-14 crash data.

3. HSIP-eligible crash clusters are defined by MassDOT as crash clusters that rank within the top five percent of crash clusters for each Regional Planning Agency, based on the EDPO index. In the Boston region the 921 intersections in the top five percent dor area clusters with a minimum EDPO value of 42.

Source: Central Transportation Planning Staff.

#### **APPENDIX A**

#### Pedestrian Report Card Assessment

- 1. Route 1A from Cherry Street to Arbor Street/Friend Court, Wenham
- 2. Route 126 and Maple Street, Bellingham





Central Transportation Planning Staff (CTPS) to the Boston Region MPO: www.ctps.org | 857.702.3700 | ctps@ctps.org

Ryan Hicks, Congestion Management Process Manager: www.ctps.org/cmp | 857.702.3661 | rhicks@ctps.org

Casey Claude, Bicycle and Pedestrian Program Manager: www.ctps.org/livability | 857.702.3707 | cclaude@ctps.org

# Pedestrian Report Card Assessment (PRCA):

# **Roadway Segment**

#### **Roadway Segment Location**

Route 1A from Cherry St. to Arbor St./Friend Ct.

Grading Categories	Score	Rating			
Safety	2.4	Good			
System Preservation	N/A	Poor			
Capacity Management and Mobility	2.16	Fair			
Economic Vitality	1.5	Poor			
Transportation Equity					

High Priority Area	
Moderate Priority Area	
Not a Priority Area	$\checkmark$

**Category Ratings** Good: Score of 2.3 or more (maximum 3.0) Fair: Score is between 1.7 and 2.3 Poor: Score is 1.7 or less (minimum 0)

# Grading Categories: Scoring Breakdown **Roadway Segment**

Capacity	Managemen	t and Mobility
	$\sim$	

Performance Measure	Weight	Rating	Weighted Score
Sidewalk Presence	3	Fair	6
Crossing Opportunities	2	Good	6
Walkway Width	1	Poor	1
Total	6		13

## **Economic Vitality**

Performance Measure	Weight	Rating	Weighted Score
Pedestrian Volumes	1	Fair	2
Adjacent Bicycle Accommodations	1	Poor	1
Total	2		3

Category rating = total rating/total weight Rating Score: Good = 3 Fair = 2 Poor = 1

Safety						
Performance Measure	Weight	Rating	Weighted Score			
Pedestrian Crashes	3	Good	9			
Pedestrian-Vehicle Buffer	1	Poor	1			
Vehicle Travel Speed	1	Fair	2			
Total	5		12			

#### **System Preservation**

Performance Measure	Rating
Sidewalk Condition	Poor

### **Transportation Equity Priority**

Area Condition	Yes/No
Environmental Justice zone?	No
School or college within one-quarter mile?	Yes
More than 8.9% of population older than 75 years?	No
More than 27.5% of households do not own a vehicle?	No

Category Ratings Good: Score of 2.3 or more (maximum 3.0) Fair: Score is between 1.7 and 2.3 Poor: Score is 1.7 or less (minimum 0)

# **Detailed Performance Measure Information: Roadway Segment**

Goal	Performance Measure	Features of Analyzed Locations		
	Sidewalk Presence	Sidewalk is present on one side of the street		
Mobility	Crossing Opportunities	2 crossing opportunities/0.2 miles =10 crosswalks per mile		
	Walkway Width	4-foot wide sidewalks		
Economic Vitality	Pedestrian Volumes	15 pedestrians per hour		
	Adjacent Bicycle Accommodations	none		
	Pedestrian Crashes	Not in HSIP cluster		
Safety	Pedestrian-Vehicle Buffer	3 feet buffers		
	Vehicle Travel Speed	32 mph		
System Preservation	Sidewalk Condition	Sidewalks are in poor condition		





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Ryan Hicks, Congestion Management Process Manager: www.ctps.org/cmp | 857.702.3661 | rhicks@ctps.org

Casey Claude, Bicycle and Pedestrian Program Manager: www.ctps.org/livability | 857.702.3707 | cclaude@ctps.org

# Pedestrian Report Card Assessment (PRCA): Intersection

#### **Intersection Location**

Route 126 and Maple St.

Grading Categories	Score	Rating
Safety	1.87	Fair
System Preservation	N/A	Poor
Capacity Management and Mobility	1.57	Poor
Economic Vitality	N/A	Fair

#### **Transportation Equity**

	-
High Priority Area	
Moderate Priority Area	
Not a Priority Area	$\checkmark$

Category Ratings Good: Score of 2.3 or more (maximum 3.0) Fair: Score is between 1.7 and 2.3 Poor: Score is 1.7 or less (minimum 0)

# Grading Categories: Scoring Breakdown Intersection

pacity Management and Mehilit

Capacity Manageme	ant an		Sinty
Performance Measure	Weight	Rating	Weighted Score
Pedestrian Delay	3	Poor	3
Sidewalk Presence	2	Fair	4
Curb Ramps	1	Fair	2
Crossing Opportunities	1	Fair	2
Total	7		11

Performance Measure	Rating
Pedestrian Volumes	Fair

Category rating =	total rating/total weight
Rating Score:	
Good = 3	
Fair = 2	
Poor = 1	

Safety												
Performance Measure	Weight	Rating	Weighted Score									
Sufficient Crossing Time (Index)	3	Poor	3									
Pedestrian Crashes	3	Good	9									
Pedestrian Signal Presence	1	Poor	1									
Vehicle Travel Speed	1	Fair	2									
Total	8		15									

#### System Preservation

Performance Measure	Rating
Sidewalk Condition	Poor

## **Transportation Equity Priority**

Area Condition	Yes/No
Environmental Justice zone?	No
School or college within a one-quarter mile?	Yes
More than 8.9% of population older than 75 years?	No
More than 27.5% of households do not own a vehicle?	No

Category Ratings Good: Score of 2.3 or more (maximum 3.0) Fair: Score is between 1.7 and 2.3 Poor: Score is 1.7 or less (minimum 0)

## **Detailed Performance Measure Information: Intersection**

Goal	Performance Measure	Features of Analyzed Locations
	Pedestrian Delay	Estimated cycle length = 150 seconds Estimated pedestrian walk/flashing don't walk time = 12 seconds Estimated pedestrian delay = 63.48 seconds
Mobility	Sidewalk Presence	Sidewalks present on all approaches
	Curb Ramps	Curb ramps are present on 2 of 3 approaches
	Crossing Opportunities	Crosswalks at 2 of 3 approaches
Economic Vitality	Pedestrian Volumes	Estimated 5 to 6 pedestrians per hour
	Sufficient Crossing Time (Index)	50 feet crossing; 12 seconds allowed; 15 seconds needed
	Pedestrian Crashes	Not in HSIP cluster
Safety	Pedestrian Signal Presence	Pedestrian signals are present on one approach. Concurrent pedestrian signal, right turn on red permitted
	Vehicle Travel Speed	31 mph
System Preservation	Sidewalk Condition	Sidewalks are in poor condition

#### **APPENDIX 6**

Support Letters from MassDOT, Wenham, and Bellingham



Charles D. Baker, Governor Karyn E. Polito, Lieutenant Governor Stephanie Pollack, Secretary & CEO Jonathan L. Gulliver, Acting Highway Administrator



Mark Abbott, Manager Traffic Analysis and Design Group Central Transportation Planning Staff Boston Region Metropolitan Planning Organization 10 Park Plaza, Suite 2150 Boston, MA 02116-3968

August 10, 2	2017		
	$\Box$	ECEIVE	
		AUG 1 8 2017	U
		CTPS	

Dear Mr. Abbott:

I am writing on behalf of MassDOT District 4 to express our support for further traffic analysis of three intersections on Main Street (Route 1A) in Wenham. These intersections are located at Cherry Street, Monument Street and Arbor Street/Friend Court.

The District's Traffic Operations Section had recently worked with the Town on a traffic signal warrant analysis of the three intersections. It was determined that MUTCD Warrants 1, 2 and 3 (volume-related warrants) were met. Since Warrants 1A and 1B, Eight-Hour Vehicular Volume, were satisfied for each of the locations, any of them would be solid candidates for a traffic signal.

To determine the true feasibility of such a project, further study in the form of a Functional Design Report (FDR) is needed. I understand that an FDR may be eligible for funding through a FY18 UPWP study entitled "Safety and Operations at Selected Locations" being conducted by your group. The Town is committed to improving safety in this area of Route 1A and is willing to complete 25% design for a project, if selected for the study. MassDOT District 4, therefore, believes that further study of the locations should be funded and completed.

Thank you for your consideration. If you have any further questions on this matter, please contact me at (781)641-8322.

Sincerely,

Paul D. Stedman District Highway Director

JEG/gb

cc: Peter Lombardi, Wenham Town Administrator Traffic File



SENATOR BRUCE E. TARR MINORITY LEADER First Essex and Middlesex The Commonwealth of Massachusetts

MASSACHUSETTS SENATE Office of the Minority Leader

AUG 1 4 2017 STATE HOUSE, ROOM 308 Boston, MA 02133-1053 Tel. (617) 722-1600 FAX: (617) 722-1310

August 1, 2017

Bruce.Tarr@MAsenate.gov www.MAsenate.gov

Mark Abbott, Manager Traffic Analysis and Design Group Central Transportation Planning Staff Boston Region Metropolitan Planning Organization Ten Park Plaza, Suite 2150 Boston, MA 02116-3968

Dear Mr. Abbott,

I would like to take this opportunity to express my strong support for the Town of Wenham. In particular, the Board of Selectmen's recent funding request for further traffic analysis regarding three intersections on Main Street in downtown Wenham, located at Cherry Street, Monument Street, and Arbor Street / Friend Court.

Given that the Main Street corridor (Route 1A) is a state road, the town worked with MassDOT District 4 Traffic Operations to complete a traffic signal warrant analysis earlier this year, which resulted in positive findings. With that, I note that all three intersections on Main Street meet the MUTCD Signal Warrants 1, 2, and 3. Noting such, I believe the relative data sufficiently satisfies Warrant 1A and 1B for Eight-Hour Vehicular traffic, with any of these locations satisfying the requirements for signal installation.

Considering such, the town is in need of assistance in determining project feasibility, specifically a Functional Design Report (FDR), which may be funded through a FY18 UPWP Study (Safety and Operations at Selected Locations). When considering the town's demonstrated commitment to addressing public safety concerns related to traffic volume, together with the number of motor vehicle crash incidents in these locations on Route 1A, I firmly believe a comprehensive operational and safety analysis of these three intersections can/should be funded and completed.

I further note, if your office is able to support/fund conducting an FDR, the town agrees to be responsible for completing a 25% design to continue to move forward with this project. This, together with the town's ongoing efforts is just another example of their demonstrated and genuine commitment to public safety.

Accordingly, I seek your careful consideration of the Town of Wenham's request for FDR funding. Thank you for such, and please don't hesitate to contact me directly should you have any questions.

Sincerely,

Bruce E. Tarr State Senator Minority Leader



# Town of Wenham

Town Hall 138 Main Street Wenham, MA 01984

 Selectmen / Town Administrator

 TEL 978-468-5520
 FAX 978-468-8014

2017 CTPS

August 1, 2017

Mark Abbott Manager, Traffic Analysis and Design Group Central Transportation Planning Staff Boston Region Metropolitan Planning Organization Ten Park Plaza, Suite 2150 Boston, MA 02116-3968

Dear Mr. Abbott,

I am writing on behalf of the Board of Selectmen to express our strong support for further traffic analysis regarding three intersections on Main Street in downtown Wenham, located at Cherry Street, Monument Street, and Arbor Street / Friend Court. Since the Main Street corridor is also a state roadway, Route 1A, we worked with MassDOT District 4 Traffic Operations to complete a traffic signal warrant analysis earlier this year.

The attached findings from that report show that these three intersections on Main Street all meet the MUTCD Signal Warrants 1, 2, and 3. Since the data satisfies Warrant 1A and 1B for Eight-Hour Vehicular Volume, our understanding is that any of these locations would be strong candidates to have a signal installed. However, the Town now needs assistance in completing the next step to determine the feasibility of this project, a Functional Design Report (FDR).

According to our Town Administrator, this project may be eligible for funding through a FY18 UPWP study entitled "Safety and Operations at Selected Locations". Given the community's ongoing public safety concerns about traffic volume and accidents along this corridor on Route 1A, we hope that your office is able to support conducting an FDR so that a comprehensive operational and safety analysis of these three intersections can be completed. We understand that, if funded, conceptual alternatives would be included in the scope of work, but that the Town would then be responsible for completing 25% design to continue to move forward with this project.

Thank you for your consideration. Please contact our Town Administrator, Peter Lombardi, at 978-468-5520 x. 2 or plombardi@wenhamma.gov if you have any further questions.

Best regards,

Jack Wilhelm Chair, Wenham Board of Selectmen



## BELLINGHAM PLANNING DEPARTMENT

10 MECHANIC STREET BELLINGHAM, MASSACHUSETTS 02019 (508) 657-2892 Plan-zone@bellinghamma.org

October 17, 2017

Mark Abbot Metropolitan Planning Organization Central Transportation Planning Staff State Transportation Building 10 Park Plaza, Suite 2150 Boston, MA 02116

Re: Technical Assistance Request - Roadway Intersection Redesign - Hartford Avenue and Maple Street

The Town of Bellingham respectfully requests technical assistance from the Central Transportation Planning Staff for the intersection redesign of Hartford Avenue and Maple Street.

Hartford Avenue at Exit 18 hosts a large inventory of commercial uses and, moving eastward, a dense residential area. In addition, a public elementary school, which services all of North Bellingham, and a small community center are located at the intersection of Hartford Avenue and Maple Street.

The intersection of Hartford Avenue and Maple Street operates well for standard motorized vehicles. However, it is over burdened by commercial vehicles. This cannot be avoided due to the proximity to I-495 and the Town's desire to retain significant acreage of industrial zoned parcels along Maple Street (See attached Zoning Map). Current industrial uses along Maple Street consist of a power plant, multiple warehouses exceeding 600,000 square feet of space, and large scale mulch and lumber hauling and production. These bring with them numerous trips by large tractor trailers.

Unfortunately this intersection is severally undersized to function properly for its desired use. The Town has recognized the need to upgrade Maple Street in order to sufficiently maintain industrial uses along this corridor. Through a public/private partnership, the town of Bellingham has begun the redesign process for the southern intersection of Maple Street and Route 140 and will be investing over \$2 Million dollars during the improvement and construction process. In a separate improvement project, the Town has invested over \$1 Million dollars to repave and correct drainage in a large section of Maple Street to better service the zoned uses. Improvement of the Hartford Avenue and Maple Street intersection is an important step in the improvement process to properly upgrade Maple Street to adequately service the industrial uses along this road and to allow large vehicles to access Route I-495 as quickly and safely as possible.

Town Officials will be available to assist and offer comments during the design process and to coordinate any public meetings that are required. Please do not hesitate to contact us if more information is necessary.

James S. Kupfer, MPA, AICP Town Planner/ Zoning Compliance Officer 10 Mechanic Street Bellingham, MA 02019 Phone: 508-657-2893 jkupfer@bellinghamma.org

Donald F. DiMartino DPW Director 26 Blackstone Street Bellingham, MA 02019 Phone - 508-966-5813 DDiMartino@bellinghamma.org



#### Town of Bellingham BOARD OF SELECTMEN

10 Mechanic Street Bellingham, Massachusetts 02019 Tel: 508-966-5800 \* Fax: 508-966-4425

November 6, 2017

Mark Abbot Metropolitan Planning Organization Central Transportation Planning Staff State Transportation Building 10 Park Plaza, Suite 2150 Boston, MA 02116

Re: Technical Assistance Request – Roadway Intersection Redesign – Hartford Avenue and Maple Street

Dear Mr. Abbot:

The Town of Bellingham Board of Selectmen is writing to express our enthusiastic support for the Town's proposal "Roadway Intersection Redesign – Hartford Avenue and Maple Street".

This intersection is severely undersized to function properly for its desired use. The Town has recognized the need to upgrade Maple Street in order to sufficiently maintain industrial uses along this corridor as well as unlock future investment. Through a public/private partnership, the town of Bellingham has begun the redesign process for the southern intersection of Maple Street and Route 140 and will be investing over \$2 Million dollars during the improvement and construction process. In a separate improvement project, the Town has invested over \$1 Million dollars to repave and correct drainage in a large section of Maple Street to better service the zoned uses. Improvement of the Hartford Avenue and Maple Street intersection is an important step in the improvement process to properly upgrade Maple Street to adequately service the industrial uses along this road and to allow large vehicles to access Route I-495 as quickly and safely as possible.

The support of the Metropolitan Planning Organization and the Central Transportation Planning Staff will be critical to following through on this important project. Thank you in advance for considering our proposal.

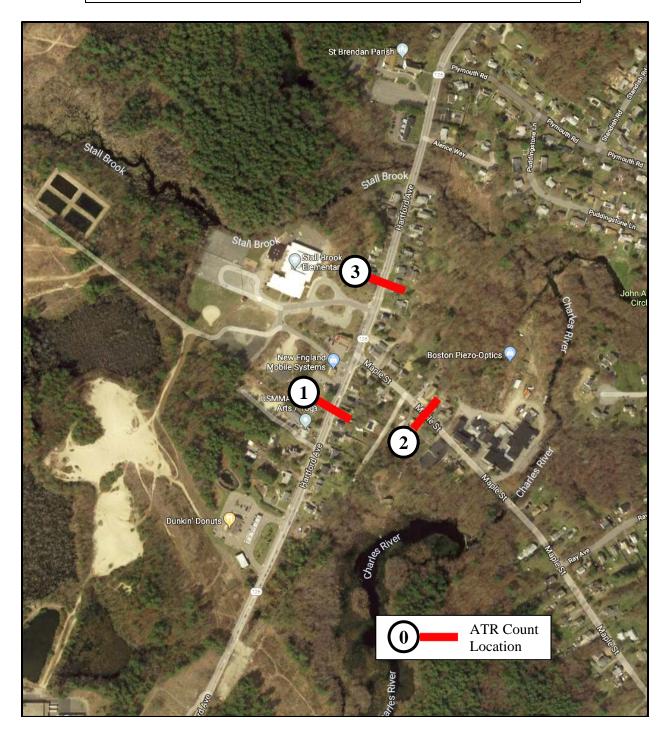
Sincerely,

Michael J. Soter, Chairman Board of Selectmen Appendix B: Traffic Data Collection

## Hartford Avenue (Route 126) in Bellingham Turning Movement Count (TMC) Locations



## Hartford Avenue (Route 126) in Bellingham Automatic Traffic Recorder (ATR) Locations



Study NameBellingham - Route 126 and Maple Street TM1 TMCStart DateWednesday, January 31, 2018 6:00 AMEnd DateWednesday, January 31, 2018 6:00 PM Site Code

#### **Report Summary**

and the second second second			T. HONE	South	bound			Content	- AN	West	bound					North	bound					East	bound		-Don to bar			Crossv	valk	And An And
Time Period	Class.	R	Т	L	U	1	0	R	Т	L	U	1	0	R	Т	L	U	I	0	R	T	L	U		0	Total		Bicycles on Crosswalk	Pedestrians	5 Total
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	0	0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	0%	
6:00 AM - 9:00 AM	Cars	0	354	47	0	401	637	147	0	115	0	262	73	26	490	7	1	524	477	7	0	0	0	7	7	1194	E	0	0	0
One Hour Peak	%	0%	78%	71%	0%	78%	79%	85%	0%	82%	0%	83%	70%	67%	78%	78%	100%	77%	79%	78%	0%	0%	0%	70%	78%	79%		0%	0%	
7:15 AM - 8:15 AM	Light Goods Vehicles	0	71	14	0	85	122	23	0	23	0	46	25	11	99	1	0	111	96	2	0	0	0	2	1	244	S	0	0	0
	%	0%	16%	21%	0%	16%	15%	13%	0%	16%	0%	15%	24%	28%	16%	11%	0%	16%	16%	22%	0%	0%	0%	20%	11%	16%		0%	0%	
	Buses	0	3	1	0	4	2	0	0	0	0	0	2	1	2	0	0	3	3	0	0	0	0	0	0	7	W	0	0	0
	%	0%	1%	2%	0%	1%	0%	0%	0%	0%	0%	0%	2%	3%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	0%	
	Single-Unit Trucks	0	16	4	0	20	31	3	0	1	0	4	4	0	28	0	0	28	17	0	0	0	0	0	0	52		0	0	0
	%	0%	4%	6%	0%	4%	4%	2%	0%	1%	0%	1%	4%	0%	4%	0%	0%	4%	3%	0%	0%	0%	0%	0%	0%	3%				
	Articulated Trucks	0	7	0	0	7	10	0	0	2	0	2	1	1	9	1	0	11	9	0	0	1	0	1	1	21				
	%	0%	2%	0%	0%	1%	1%	0%	0%	1%	0%	1%	1%	3%	1%	11%	0%	2%	1%	0%	0%	100%	0%	10%	11%	1%				
	Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0				
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%				
	Total	0	451	66	0	517	802	173	0	141	0	314	105	39	628	9	1	677	602	9	0	1	0	10	9	1518				
	PHF	0	0.93	0.75	0	0.9	0.93	0.86	0	0.9	0	0.93	0.8	0.75	0.92	0.45	0.25	0.91	0.94	0.56	0	0.25	0	0.5	0.45	0.96				
	Approach %					34%	53%					21%	7%					45%	40%					1%	1%					
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	0	2	2
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	100%	
2:00 PM - 6:00 PM	Cars	0	528	163	0	691	562	74	0	66	0	140	266	103	488	4	0	595	603	9	0	0	0	9	4	1435	E	0	0	0
One Hour Peak	%	0%	80%	82%	0%	81%	86%	84%	0%	83%	0%	83%	83%	85%	86%	100%	0%	86%	81%	100%	0%	0%	0%	100%	100%	83%		0%	0%	
4:30 PM - 5:30 PM	Light Goods Vehicles	0	114	36	0	150	85	14	0	13	0	27	52	16	71	0	0	87	127	0	0	0	0	0	0	264	S	0	0	0
	%	0%	17%	18%	0%	18%	13%	16%	0%	16%	0%	16%	16%	13%	12%	0%	0%	13%	17%	0%	0%	0%	0%	0%	0%	15%		0%	0%	
	Buses	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	W	0	1	1
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	18	0%	100%	State of the second
	Single-Unit Trucks	0	12	0	0	12	8	0	0	1	0	1	2	2	8	0	0	10	13	0	0	0	0	0	0	23		0	3	3
	%	0%	2%	0%	0%	1%	1%	0%	0%	1%	0%	1%	1%	2%	1%	0%	0%	1%	2%	0%	0%	0%	0%	0%	0%	1%				
	Articulated Trucks	0	1	1	0	2	2	0	0	0	0	0	1	0	2 .	0	0	2	1	0	0	0	0	0	0	4				
	%	0%	0%	1%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%				
	Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0				
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%				
	Total	0	656	200	0	856	657	88	0	80	0	168	321	121	569	4	0	694	745	9	0	0	0	9	4	1727				
	PHF	0	0.87	0.88	0	0.89	0.88	0.71	0	0.83	0	0.76	0.96	0.92	0.85	0.5	0	0.88	0.91	0.45	0	0	0	0.45	0.5	0.91				
	Approach %					50%	38%					10%	19%					40%	43%				1.03	1%	0%					
	a faill an new log of the			12 1 1	Saldue.	e line la		Servin 1	Rigtor			pellion:	18.0.0					20 martin		1 P P R P						<b>HEM MUS</b>	1.1.1.1.1			antifica .

# Study NameBellingham - Route 126 and Stall Brook School Driveway TM2 TMCStart DateWednesday, January 31, 2018 6:00 AMEnd DateWednesday, January 31, 2018 6:00 PMSite Code

#### **Report Summary**

			Southbound					Northbound					E	astbou	nd			Crosswalk			
Time Period	Class.	R	Т	U	I	0	Т	L	U	1	0	R	L	U	Ĵ.	0	Total		<b>Bicycles on Crosswalk</b>	Pedestrians	Total
Peak 1	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	0	0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	0%	
6:00 AM - 9:00 AM	Cars	9	390	0	399	642	642	20	0	662	392	2	0	0	2	29	1063	S	0	0	0
One Hour Peak	%	90%	79%	0%	79%	78%	78%	95%	0%	79%	79%	100%	0%	0%	100%	94%	79%		0%	0%	
7:00 AM - 8:00 AM	Light Goods Vehicles	1	77	0	78	132	132	1	0	133	77	0	0	0	0	2	211	W	0	0	0
	%	10%	16%	0%	16%	16%	16%	5%	0%	16%	16%	0%	0%	0%	0%	6%	16%		0%	0%	
	Buses	0	3	0	3	1	1	0	0	1	3	0	0	0	0	0	4		0	0	0
	%	0%	1%	0%	1%	0%	0%	0%	0%	0%	1%	0%	0%	0%	0%	0%	0%				
	Single-Unit Trucks	0	17	0	17	33	33	0	0 ·	33	17	0	0	0	0	0	50				
	%	0%	3%	0%	3%	4%	4%	0%	0%	4%	3%	0%	0%	. 0%	0%	0%	4%				
	Articulated Trucks	0	5	0	5	10	10	0	0	10	5	0	0	0	0	0	15				
	%	0%	1%	0%	1%	1%,	1%	0%	0%	1%	1%	0%	0%	0%	0%	0%	1%				
	Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0				
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%				
	Total	10	492	0	502	818	818	21	0	839	494	2	0	0	2	31	1343				
	PHF	0.62	0.87	0	0.87	0.97	0.97	0.66	0	0.96	0.87	0.5	0	0	0.5	0.65	0.95				
	Approach %				37%	61%				62%	37%				0%	2%					
Peak 2	Motorcycles	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	N	0	0	0
Specified Period	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%		0%	0%	
2:00 PM - 6:00 PM	Cars	4	681	1	686	563	557	5	0	562	697	16	5	0	21	9	1269	S	0	0	0
One Hour Peak	%	100%	81%	100%	81%	86%	86%	100%	0%	86%	81%	89%	100%	0%	91%	100%	83%		0%	0%	
4:30 PM - 5:30 PM	Light Goods Vehicles	0	145	0	145	79	79	0	0	79	147	2	0	0	2	0	226	W	0	0	0
	%	0%	17%	0%	17%	12%	12%	0%	0%	12%	17%	11%	0%	0%	9%	0%	15%		0%	0%	
	Buses	0	1	0	1	0	0	0	0	0	1	0	0	0	0	0	1		0	0	0
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%				
	Single-Unit Trucks	0	13	0	13	9	9	0	0	9	13	0	0	0	0	0	22				
	%	0%	2%	0%	2%	1%	1%	0%	0%	1%	2%	0%	0%	0%	0%	0%	1%				
	Articulated Trucks	0	2	0	2	2	2	0	0	2	2	0	0	0	0	0	4				
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%				
	Bicycles on Road	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0				
	%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%				
	Total	4	842	1	847	653	647	5	0	652	860	18	5	0	23	9	1522				
	PHF	0.33	0.9	0.25	0.9	0.9	0.89	0.42	0	0.89	0.9	0.75	0.42	0	0.64	0.56	0.9				
	Approach %				56%	43%				43%	57%	Mar BIL:			2%	1%	E .				
		14	and parts	(Suppl)	A TALE	19-12-26					Selfa 2		4. 1	and the second		a ar			1. 1. 1. 11年 市市市		

MassDOT Highway Division WEEKLY SUMMARY FOR LANE Starting: 2/12/2018

Page: 3

STA. 1

TOTAL

File: SPDC1.prn City: BELLINGHAM County: SPEED NB&SB

Site Reference: 180040000798 Site ID: 00000000101 Location: ROUTE 126 SOUTH OF MAPLE ST. Direction: ROAD TOTAL

TIME	12	13	14	THU 15	WKDAY AVG	SAT	SUN	WEEK AVG	TOTAL
01:00					62			62	188
02:00		66 27	60 36	35	32			32	98
03:00		28	22	24	24			24	74
04:00		41		44	46			46	138
05:00			126	115	127			127	382
06:00		422	427	405	418			418	1254
07:00		976 1192	962	938	958			958	2876
08:00		1192	1211	1153	1185			1185	3556
09:00			1263	1220	1242			1242	3726
10:00		-	1039		1009			1009	2019
11:00			988		978			978	1957
12:00	1027		1120		1060			1060	3181
13:00	1091	1140	1128		1119			1119	3359
14:00	1117	1119	1152		1129			1129	3388
15:00	1232	1253	1229		1238			1238	3714
			1402		1415			1415	4245
			1393		1412			1412	4238
18:00	1423	1426	1439		1429			1429	4288
19:00	1154	1245 860	1176		1191			1191	3575
20:00	743	860	829		810			810	2432
21:00	549	657	567		591			591	1773
	309				368			368	1104
23:00	197	1/9	238		204 132			132	614
24:00	114	141	143		132			132	920
TOTALS					18179				
% AVG WKDY	64.4	101.3	101.4	21.9					
	64.4		101.4						
AM Times	12:00	09:00	09:00					09:00	(ii)
AM Peaks	1027	1243	1263	1220	1242			1242	
PM Times	18:00	17:00	18:00		18:00			18:00	
PM Peaks	1423	1471	1439		1429			1429	

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COMB AWD 18179

FAC 1.00 CONB ADT 18,200

#### MassDOT Highway Division WEEKLY SUMMARY FOR LANE 1 Starting: 2/12/2018

STA.INB

Site Reference: 180040000798 Site ID: 00000000101 Location: ROUTE 126 SOUTH OF MAPLE ST. Direction: NORTH File: SPDC1.prn City: BELLINGHAM County: SPEED NB&SB

	MON 12	13	14	15		WKDAY AVG			WEEK AVG	
01:00			27			28				86
		10	15	17		14				42
03:00		15	9	14		12			12	38
04:00		24 98	31 92	25 77		26			26	80
05:00						89		35	89	267
06:00		291	294	285		290			290	
07:00		688	671	659		672			672	2018
08:00		630	623	603		618			618	1856
09:00		637	656	647		646			646	1940
10:00		489	524			506			506	1013
11:00		479	516			497			497	
12:00	536	515	577			542			542	1628
	541	548	577			555			555	1666
14:00	609	584	607			600			600	1800
15:00	599	599	601			599			599	1799
16:00	633	687	671			663			663	1991
17:00	656	701	668			675			675	2025
	744		710			727			727	2182
	557		590			582			582	1747
20:00	395	440	398			411			411	1233
21:00 22:00	323	386	286			331			331	995
22:00	187	214	256			219			219	<b>VJ</b> /
23:00	105	95	116			105			105	
24:00	45	48	64			52			52	157
TOTALS	5930	9537	9579	2355	0	9459	0	0	9459	27401
% AVG WKDY	62.6	100.8	101.2	24.8						
	62.6			24.8						
AM Times	12:00	07:00	07:00	07:00		07:00			07:00	
AM Peaks	536	688	671	659		672			672	
PM Times						18:00			18:00	
PM Peaks	744	728	710		·	727			727	

#### MassDOT Highway Division WEEKLY SUMMARY FOR LANE 2 Starting: 2/12/2018

STA. I SB

File: SPDC1.prn City: BELLINGHAM County: SPEED NB&SB

Site Reference: 180040000798 Site ID: 00000000101 Location: ROUTE 126 SOUTH OF MAPLE ST. Direction: SOUTH

TIME		TUE 13		THU 15	FRI	WKDAY AVG	SAT	WEEK AVG	
01:00		35	33	34		34		 34	
02:00		17	21	18		18		-	56
03:00			13	10		12		12	36
04:00			22	19		19		19	58
05:00		43	34	38		38		38	115
06:00		131	133	120		128		129	384
07:00		288	291	279		286		286	858
08:00		562	588	550		566		566	1700
09:00		606	607	573		595		595	1786
10:00		491	515			503		503	1006
11:00		490	472			481		481	962
12:00	491 550	519	543			517		517	1553
13:00	550		551			564		564	1693
	508	535	545			529		529	1588
	633	654	628			638		638	1915
16:00	762	761	731			751 737		751 737	2254 2213
17:00	718	770 698	725 729			702		702	2213
18:00 19:00	679 597	645	586			609	3,2	609	1828
20:00	348	420	431			399		399	1199
	226		281			259			778
	122		175			149		149	
23:00	92	84	122			99		99	298
24:00	92 69	93	79			80			241
TOTALS				1641	0	8713	0	8713	
<pre>% AVG WKDY</pre>	66 B	101 0	101 6	10 0					
& AVG WEEK	66.5			18.8					
AM Times	12:00	09:00	09:00	09:00		09:00		09:00	
				573				595	
PM Times	16:00	17:00	16:00			16:00		16:00	
PM Peaks	762	770	731			751		751	

MassDOT Highway Division WEEKLY SUMMARY FOR LANE Starting: 2/12/2018

TOTAL

STA.2

Site Reference: 180040000668 Site ID: 00000000203 Location: MAPLE STREET EAST OF RTE. 126 Direction: ROAD TOTAL File: SPDC2.prn City: BELLINGHAM County: SPEED EB&WB

TIME		13	14	THU 15		WKDAY AVG		SUN	WEEK AVG	TOTAL
01.00									47	141
01:00 02:00		44	47 8	20		47 14			47	141 44
02:00		37	8	9		14				55
04:00		32	33	36		े <u>२२</u>			33	101
05:00		76	93	71		80			80	240
06:00		243	267	242		250			250	752
07:00		669	671	620		653			653	1960
08:00		669 910	863	896		889			889	2669
09:00		821	803	776		800			800	2400
10:00			632			581			581	1162
11:00		476	608			542			542	1084
	614	570	594			592			592	1778
13:00	641	663 578	691			665			665	1995
14:00		578	678			620			620	1861
15:00	763	756	771			763			763	2290
16:00	953	912	929			931			931	2794
	919		1004			976			976	2930
18:00	1074		957			997			997	2992
19:00 20:00 21:00	839	961 810 456	781			810			810	2430
20:00	384	456	432			424			424	1272
21:00	306	439	317			354			354	1062
22:00	175	293	281			249				749
	136					141			141	
24:00	80	75	82			79			79	237
TOTALS				2721	0	11508	0	0	11508	33422
% AVG WKDY	65	99.9	101.7	23.6						
% AVG WEEK	65	99.9	101.7	23.6						
AM Times	12:00	08:00	08:00	08:00		08:00			08:00	
AM Peaks	614	910	863	896		889			889	
PM Times						18:00			18:00	
PM Peaks	1074	1007	1004			997			997	

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COMB AND 11508 FAC 1.03 COMB ADT 11,800

#### MassDOT Highway Division WEEKLY SUMMARY FOR LANE 1 Starting: 2/12/2018

STA.2EB

Site Reference: 180040000668 Site ID: 00000000203 Location: MAPLE STREET EAST OF RTE. 126 Direction: EAST File: SPDC2.prn City: BELLINGHAM County: SPEED EB&WB

TIME		13	14	THU 15	FRI	WKDAY AVG	SAT	SUN	WEEK AVG	TOTAL
01:00		19	25	25		23			23	69
02:00		6 18	3	8		5			5	17
03:00		18	3 16	5 21		8 18			8	26 56
04:00		13	16	21					18 47	20
05:00		47	56	39		47				
06:00		127	153	125		135				405
07:00		356	345	320		340			340	1021
08:00		469	455	465		463			463	1389
09:00		431	407	402		413			413	1240 587
10:00		275	312			293				
11:00	~	224	297			260				521
	295					294				883 967
	321		315			322			322	
14:00	284	273	326			294			294	
15:00	368 417	362	359			363			363	1089
16:00	417	431	427			425			425	1275
17:00	426	476	476			459			459	1378
	503	445	457			468			468	1405
	397	360	345			367			367	1102
20:00	189	214	211			204			204	614 498
21:00	145 82 61	210	143			166			166 113	490
22:00	82	135	124			113			63	
23:00	61	60	68			63				
24:00	36	35	35			35			35	100
TOTALS				1410	0	5578	0	0	5578	16203
% AVG WKDY	63.1	100.5	101.5	25.2						
<b>% AVG WEEK</b>	63.1	100.5	101.5	25.2						
AM Times	12:00	08:00	08:00	08:00		08:00				
AM Peaks	295	469	455	465		463			463	
PM Times						18:00			18:00	
PM Peaks	503	476	476			468			468	

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#### MassDOT Highway Division WEEKLY SUMMARY FOR LANE 2 Starting: 2/12/2018

STA.2WB

Site Reference: 180040000668 Site ID: 00000000203 Location: MAPLE STREET EAST OF RTE. 126 Direction: WEST File: SPDC2.prn City: BELLINGHAM County: SPEED EB&WB

TIME		13	14	THU 15		WKDAY AVG			WEEK AVG	
01:00		25	22	25		24 9 9				
02:00		9	5	13		9			24 9 9	27
03:00		19	6	4		9			9	29
04:00		13	17	15		15			15	
05:00		29 116	37	32		32			32	98
06:00		116	114	117		115			115	347
07:00		313 441 390	326	300		313			313	939
08:00		441	408	431		426			426	1280
09:00		390	396	374		386			386	1160
10:00		255	320			287			287	575
11:00		252	311			281			281	563
12:00	319	287	289			298			298	895
13:00	320	332	376			342			342	1028
14:00	320 321	305	352			326			326	978
15:00	395	394	412			400			400	1201
	536	481	502			506			506	1519
17:00	493	531	528			517			517	1552
18:00	571	516	500			529			529	1587
19:00	442	450	436			442			442	1328
20:00	195	242	221			219			219	
21:00	442 195 161	229	174			188			188	564
22:00	93	158	157			136			136	408
23:00	75	69	91			78			78	235
24:00	44	40	47			43			43	131
TOTALS	3965	5896	6047	1311	0	5921	0	0	5921	17219
8 AVG WKDY	66.9	99.5	102.1	22.1						
	66.9			22.1						
AM Times	12:00	08:00	08:00	08:00		08:00			08:00	
AM Peaks	319	441	408	431		426			426	
PM Times						18:00			18:00	
PM Peaks	571	531	528			529			529	

MassDOT Highway Division WEEKLY SUMMARY FOR LANE 1 Starting: 2/12/2018

STA.3 SB

File: CL302.prn City: BELLINGHAM County: CLASS SB

Site Reference: 180040000400 Site ID: 00000000302 Location: ROUTE 126 NORTH OF MAPLE ST. Direction: SOUTH

TIME		13	WED 14			WKDAY AVG	SAT	SUN	WEEK AVG	TOTAL
01:00						82			82	246
02:00		94 37	67 36	48		40			40	121
03:00		46	22	30		32			32	98
04:00		72	58	69		66			66	199
05:00		205	144	168		172			172	517
06:00		465	429	430		441			441	1324
07:00		622	880	624		708			708	2126
08:00		636	949	651		745			745	2236
09:00		688	927	645		753			753	2260
10:00		650	815			732			732	1465
11:00		763	787			775			775	1550
12:00	574	788	788			716			716	2150
13:00	694 677	847	762			767			767	2303
14:00	677	872	769			772			772	2318
15:00		966	685			785			785	2355
16:00	731	1016	683			810			810	2430
17:00		990	746			825			825	2476
18:00	703	1035	745			827			÷ – ·	2483
19:00	665	905	780			783			783	2350
20:00	537	100	655			642			642	1927
21:00	469	580	512			520			520	
	340		421			371			371	
23:00	243	181	269			231				693
24:00	146	139	176			153			153	461
TOTALS	7223	13685	13105	2750	0	12748	0	0	12748	36763
<pre>% AVG WKDY % AVG WEEK</pre>	56.6 56.6			21.5 21.5						
AM Times AM Peaks	12:00 574	12:00 788	08:00 949	08:00 651		11:00 775			11:00 775	
PM Times PM Peaks	17:00 740					18:00 827			18:00 827	

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#### MassDOT Highway Division WEEKLY SUMMARY Starting:2/12/2018

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11

Site Reference: 180040000798 Site ID: 00000000101 Location: ROUTE 126 SOUTH OF MAPLE ST. Direction: ROAD TOTAL STAIL

TOTAL

File: SPDC1.prn City: BELLINGHAM County: SPEED NB&SB

TIME	MON 12	TUE 13	WED 14	THU 15	FR	I	SF	T	st	JN	WK	TOT	WK	AVG
Lane 3	am pm	am pm	am pm	am pm	am	pm	am	pm	am	pm	am	pm	am	pm
		279       190         299       195         315       182         347       162         282       118         258       103         254       95         235       90         233       76         223       66         251       42         246       40         241       46         261       41         255       30         287       24	250         58           253         61           240         50           263         47           229         38           322         -23           306         35	14 17 19 12 17 7 7 4 9 5 6 4 4 5 6 18 15 13 26 28 48 71 70 126 138 178 234 255 282 316 300 283 312 336 289							$\begin{array}{c} 21\\ 20\\ 58\\ 39\\ 60\\ 73\\ 105\\ 144\\ 75\\ 96\\ 001\\ 828\\ 8948\\ 8948\\ 8948\\ 8948\\ 8948\\ 8948\\ 10460\\ 2474\\ 360\\ 1860\\ 999\\ 747\\ 8360\\ 7447\\ 8360\\ \end{array}$	161 142 108 126 114 78 80	286	53 47 36 42 38 26 26
TOTALS		18422	18434	3996		0		0		0		2577	18	158
AM Times AM Peaks	11:15 1027	8:15 1243	8:00 1277	8:00 1231								8:00 3748		8:00 1248
PM Times PM Peaks	17:15 1423	16:00 1484						11				15:45 4298		15:45 1431

#### MassDOT Highway Division WEEKLY SUMMARY Starting:2/12/2018

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Site Reference: 180040000798 Site ID: 00000000101 Location: ROUTE 126 SOUTH OF MAPLE ST. Direction: NORTH STA.I NB

File: SPDC1.prn City: BELLINGHAM County: SPEED NB&SB

TIME	MON 12	TUE 13	WED 14	THU 15	FRI	SAT	SUN	WK TOT	WK AVG
Lane 1	am pm	am pm	am pm	am pm	am pm	am pm	am pm	am pm	am pm
	31 22 13 123 11 138 12 128 12 147 10		128       22         137       32         128       27         138       20         113       13         175       12         151       19					$\begin{array}{cccccccccccccccccccccccccccccccccccc$	
TOTALS AM Times		9537 6:30		2355 6:30	0	0	0	27401 6:30	9437 6:30
AM Peaks	536	725	679	671				2075	690
PM Times PM Peaks	17:15 744	16:45 737	17:15 710					17:15 2182	17:15 726

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Site Reference: 180040000798 Site ID: 00000000101 Location: ROUTE 126 SOUTH OF MAPLE ST. Direction: SOUTH STA.1 SB

File: SPDC1.prn City: BELLINGHAM County: SPEED NB&SB

TIME	MON 12	TUE 13	WED 14	THU 15	FRI	SAT	SUN	WK TOT	WK AVG
Lane 2	am pm	am pm	am pm	am pm	am pm	am pm	am pm	am pm	am pm
00:15 00:30 00:45 01:00 01:15 01:30 02:00 02:15 02:30 02:45 03:00 03:15 03:30 03:45 04:00 04:15 04:30 04:45 05:00 05:15 05:30 05:45 06:15 06:15 06:15 06:15 06:15 06:15 06:15 06:45 07:00 07:15 07:30 07:45 08:00 08:15 08:30 08:45 09:00 09:15 09:30 09:45 10:015 10:30 10:45 11:00 11:15 11:30 11:45	142 142 127 139 125 131 133 119 122 158 163 190 185 203 191 183 180 190 172 176 171 161 186 161 162 152 146 137 112 84 92 60 72 68 53 33 31 34 30 27 29 30 19 14 124 129 128 13 120 11	$\begin{array}{c} 9 & 153 \\ 10 & 151 \\ 10 & 149 \\ 6 & 139 \\ 9 & 136 \\ 1 & 136 \\ 4 & 124 \\ 3 & 139 \\ 3 & 145 \\ 5 & 146 \\ 4 & 185 \\ 1 & 178 \\ 1 & 217 \\ 1 & 203 \\ 9 & 159 \\ 6 & 182 \\ 8 & 213 \\ 5 & 202 \\ 11 & 183 \\ 15 & 202 \\ 11 & 183 \\ 19 & 172 \\ 14 & 188 \\ 25 & 188 \\ 34 & 150 \\ 58 & 172 \\ 52 & 172 \\ 68 & 163 \\ 81 & 168 \\ 87 & 142 \\ 126 & 126 \\ 155 & 97 \\ 157 & 117 \\ 124 & 80 \\ 146 & 69 \\ 130 & 74 \\ 194 & 75 \\ 136 & 53 \\ 135 & 41 \\ 137 & 35 \\ 109 & 40 \\ 110 & 34 \\ 113 & 33 \\ 125 & 18 \\ 123 & 17 \\ 129 & 16 \\ 113 & 33 \\ 125 & 18 \\ 123 & 17 \\ 129 & 16 \\ 113 & 33 \\ 125 & 18 \\ 123 & 17 \\ 129 & 16 \\ 113 & 33 \\ 125 & 18 \\ 123 & 17 \\ 129 & 16 \\ 113 & 33 \\ 125 & 18 \\ 123 & 17 \\ 129 & 16 \\ 113 & 33 \\ 125 & 10 \\ 127 & 21 \\ 151 & 10 \\ \end{array}$	$  \begin{array}{c} 6 & 167 \\ 4 & 128 \\ 10 & 120 \\ 13 & 136 \\ 9 & 134 \\ 3 & 140 \\ 7 & 134 \\ 2 & 137 \\ 2 & 137 \\ 2 & 137 \\ 2 & 139 \\ 4 & 147 \\ 4 & 152 \\ 3 & 190 \\ 4 & 185 \\ 3 & 173 \\ 12 & 187 \\ 3 & 186 \\ 9 & 183 \\ 8 & 190 \\ 6 & 170 \\ 11 & 182 \\ 28 & 166 \\ 25 & 203 \\ 30 & 202 \\ 50 & 158 \\ 47 & 156 \\ 61 & 163 \\ 75 & 132 \\ 108 & 135 \\ 120 & 109 \\ 143 & 117 \\ 163 & 102 \\ 162 & 103 \\ 134 & 83 \\ 134 & 67 \\ 155 & 16 \\ 162 & 23 \\ 122 & 34 \\ 122 & 36 \\ 116 & 29 \\ 112 & 23 \\ 125 & 27 \\ 116 & 25 \\ 147 & 11 \\ 155 & 16 \\ \end{array} $	8 9 8 9 12 0 4 4 2 4 4 4 4 1 1 1 1 1 1 1 1 1 1 1 1 1				$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$
TOTALS		8885						25176	
AM Times AM Peaks		8:15 606	8:45 637	8:00 589				8:00 1800	8:45 610
PM Times PM Peaks	15:00 769	14:45 783						15:00 2261	15:00 753

STA. 2

# Site Reference: 180040000668 Site ID: 00000000203 Location: MAPLE STREET EAST OF RTE. 126 Direction: ROAD TOTAL

TOTAL

File: SPDC2.prn City: BELLINGHAM County: SPEED EB&WB

TIME	MON 12	TUE 13	WED 14	THU 15	FRI	SAT	SUN	WK TOT	WK AVG
Lane 3	am pm	am pm	am pm	am pm	am pm	am pm	am pm	am pm	am pm
12:00	32 21 21 157 10 153 29 135 25 169 16	130       28         120       33         121       18         136       21         131       18         146       17         157       19	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	12 6 14 18 3 12 6 0 0 0 6 3 6 9 3 18 6 9 3 18 6 9 3 18 6 15 19 31 37 48 77 80 105 161 158 196 193 223 251 229 174 208 208 186				$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$
TOTALS AM Times		11502 7:15			-	0	0	33422 7:30	
AM Peaks	614	910	900	896				2681	892
PM Times PM Peaks	17:15 1074	17:00 1018	16:15 1004					17:00 3016	17:00 1005

Page: 1

Site Reference: 180040000668 Site ID: 00000000203 Location: MAPLE STREET EAST OF RTE. 126 Direction: EAST STA. 2 EB

File: SPDC2.prn City: BELLINGHAM County: SPEED EB&WB

TIME	MON 12	TUE 13	WED 14	THU 15	FRI	SAT	SUN	WK TOT	WK AVG
Lane 1	am pm	am pm	am pm	am pm	am pm	am pm	am pm	am pm	am pm
00:15 00:30 00:45 01:00 01:15 01:30 01:45 02:00 02:15 02:30 02:45 03:00 03:15 03:30 03:45 04:00 04:15 04:00 04:15 04:30 04:45 05:00 05:15 05:30 05:45 06:00 06:15 06:30 06:45 07:00 06:45 07:30 07:45 08:30 08:45 09:00 09:15 09:30 09:45 10:00 10:15 10:30 10:45 11:00 11:45 12:00	$\begin{array}{c} 78\\ 92\\ 76\\ 75\\ 61\\ 77\\ 69\\ 77\\ 78\\ 95\\ 93\\ 102\\ 95\\ 110\\ 109\\ 103\\ 113\\ 98\\ 109\\ 106\\ 146\\ 126\\ 114\\ 117\\ 123\\ 94\\ 86\\ 94\\ 71\\ 45\\ 34\\ 39\\ 42\\ 38\\ 38\\ 27\\ 31\\ 12\\ 21\\ 18\\ 28\\ 13\\ 10\\ 10\\ 77\\ 4\\ 75\\ 13\\ 61\\ 11\\ 82\\ 8\end{array}$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	6 4 6 9 1 5 2 0 0 0 0 3 2 4 5 1 11 3 8 11 17 20 24 39 42 63 84 74 99 103 120 132 110 88 106 109 99				$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$
TOTALS	3524	5606	5663 7:30	1410 7:15	0	0	0	16203 7:15	5551 7:15
AM Times AM Peaks	11:15 295	7:15 469	476	465				1389	462
PM Times PM Peaks	17:15 503	17:00 486	16:15 476					17:00 1442	17:00 478

Page: 2

Site Reference: 180040000668 Site ID: 00000000203 Location: MAPLE STREET EAST OF RTE. 126 Direction: WEST

STA . 2 WB

File: SPDC2.prn City: BELLINGHAM County: SPEED EB&WB

TIME	MON 12	TUE 13	WED 14	THU 15	FRI	SAT	SUN	WK TOT	WK AVG
Lane 2	am pm	am pm	am pm	am pm	am pm	am pm	am pm	am pm	am pm
00:15 00:30 00:45 01:00 01:15 01:30 01:45 02:00 02:15 02:30 03:00 03:15 03:30 03:45 04:00 04:15 04:30 04:45 05:00 05:15 05:30 05:45 06:00 05:45 06:5 06:30 06:45 07:00 07:15 07:30 07:45 08:00 08:15 08:30 08:45 09:00 09:15 09:30 09:45 10:00 10:15 10:30 10:45 11:00 11:15 11:30 11:45 12:00	76 77 86 81 69 89 85 78 92 93 98 112 127 136 149 124 130 101 131 131 153 133 125 160 137 108 95 102 65 39 38 53 48 41 47 25 29 20 24 20 34 19 11 11 80 67 8 16 78 16 78 16 78 16 78 16 78 87 87 87 87 87 87 87 87 87 87 87 87	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	6 2 8 9 2 7 4 0 0 0 3 1 2 4 2 4 2 7 7 3 7 8 14 17 24 38 38 42 77 84 97 90 103 119 102 99 87 87	0		Ο	24       275         14       253         15       247         19       253         2       228         13       270         10       253         2       227         2       279         14       308         4       338         6       361         5       385         13       394         21       379         16       406         24       369         26       395         32       382         50       421         88       386         94       385         115       395         130       417         240       331         259       295         310       285         259       195         328       167         338       133         355       163         271       160         332       152         309       145         248       107         155       122	$\begin{array}{cccccccccccccccccccccccccccccccccccc$
TOTALS AM Times	3965 11:15			7:15	0	U		7:45	7:45
AM Times AM Peaks	319	445	439	431		ŝ		1296	430
PM Times PM Peaks	17:15 571	16:30 541						17:15 1587	17:15 527

Page: 1

Site Reference: 180040000400 Site ID: 00000000302 Location: ROUTE 126 NORTH OF MAPLE ST. Direction: SOUTH STA.35B

File: SPD302.prn City: BELLINGHAM County: SPEED SB

TIME	MON 12	TUE 13	WED 14	THU 15	FI	RI	SF	T	st	JN	WK	TOT	WK	AVG
Lane 1	am pm	am pm	am pm	am pm	am	pm	am	pm	am	pm	am	pm	am	pm
	72 57 34 140 43 156 42 150 34 128 27	176       47         205       42         200       33         182       42         208       41         182       32         209       24	208       65         196       76         188       59         201       56         173       46         222       27         190       47	20 18 30 17 21 11 11 5 13 5 6 6 6 16 24 23 24 31 52 61 77 93 126 134 158 142 175 149 141 173 158 179 149 141 173 158 179 149 141 175 135 175 149 145 175 145 175 145 175 145 175 175 145 175 175 175 175 175 175 175 17							65 52 84 45 22 24 25 26 23 22 24 25 26 23 22 24 25 26 23 22 24 25 26 23 22 24 25 26 23 22 24 25 26 23 22 24 25 26 23 22 24 25 26 23 22 24 25 26 23 22 24 25 26 23 229 76 62 88 154 174 294 398 418 529 536 575 5383 575 528 3763 3733 3841 408 537 554 557 527 557 528 3773 3841 408 537 554 527 57	184 175 126 141 129 93 98	$\begin{array}{c} 21\\ 17\\ 28\\ 15\\ 7\\ 10\\ 7\\ 8\\ 8\\ 7\\ 7\\ 13\\ 20\\ 29\\ 33\\ 58\\ 71\\ 325\\ 209\\ 331\\ 58\\ 71\\ 325\\ 209\\ 331\\ 58\\ 1399\\ 1399\\ 1399\\ 1798\\ 1985\\ 1991\\ 178\\ 1985\\ 1991\\ 176\\ 1886\\ 1991\\ 176\\ 1886\\ 1992\\ 194\\ 179\\ 1886\\ 1992\\ 194\\ 179\\ 1886\\ 1992\\ 194\\ 179\\ 1886\\ 1992\\ 194\\ 179\\ 1886\\ 1992\\ 194\\ 175\\ 1886\\ 1992\\ 194\\ 175\\ 184\\ 175\\ 184\\ 175\\ 184\\ 175\\ 184\\ 175\\ 184\\ 175\\ 184\\ 175\\ 184\\ 175\\ 184\\ 175\\ 184\\ 175\\ 184\\ 175\\ 184\\ 175\\ 184\\ 175\\ 184\\ 175\\ 184\\ 175\\ 184\\ 175\\ 186\\ 196\\ 196\\ 196\\ 196\\ 196\\ 196\\ 196\\ 19$	$\begin{array}{c} 197\\ 203\\ 186\\ 179\\ 193\\ 186\\ 199\\ 193\\ 186\\ 194\\ 182\\ 006\\ 2017\\ 503\\ 2002\\ 202\\ 202\\ 202\\ 202\\ 202\\ 202\\ $
TOTALS AM Times	7223	13625 10:45		2750 8:00		0		0		0	36	650 8:00		686 10:15
AM Peaks	574	795	950	679								2296		772
PM Times PM Peaks	15:30 747	17:15 1034	17:45 792									17:00 2515		17:00 836

#### MassDOT Highway Division SPEED SUMMARY Mon 2/12/2018

Site Refere Site ID: 00 Location: I Direction: Lane: 1	0000000 ROUTE 1	0101		MAPLE	ST.	ST	A , /	NB		File: City: County	BELLIN	IGHAM	3B			
TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
								·								-
12:00	14	37	134	217	114	18	2	0	0	0	0	0	0	0	0	536
13:00	69	60	129	180	91	9	3	0	0	0	0	. 0	Ō	Ō	0	541
14:00	34	53	139	232	128	20		0	0	0	0	0	Ō	0	0	609
15:00	86	92		172	87	16	3 0	0	0	0	0	0	0	0	0	599
16:00	81	72	128	224	108	19	1	0	0	0	0	0	0	0	0	633
17:00	56	134	206	184	71	4	1	0	0	0	0	0	0	0	0	656
18:00	363	175	117	66	20	3	1 0	0	0	0	0	0	0	0	0	744
19:00	33	73	167	176	91	17	0	0	0	0	0	0	0	0	0	557
20:00	4	35	67	160	103	25	1	0 0	0	0	0	0	0	0	0	395
21:00	12	9	51	130	105	14	1 2 2	0	0	0	0	0	0	0	0	323
22:00	0	0	11	67	86	21	2	0	0	0	0	· 0	0	0	0	187
23:00	0	3	9	33	44	14	2 1	0 1	0	0 .	0	0	0	0	0	105
24:00	0	. 0	4	8	20	11	1	1	0	0	0	0	0	0	0	45
DAY TOTAL	752	743	1308	1849	1068	 191	18	1	0	0	0	0	0	0	0	5930
PERCENTS			22.1%			3.2%	0.3%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 19.9 mph 85th Percentile Speed 35.8 mph

Median Speed 29.4 mph

10 MPH Pace Speed 24 mph to 34 mph 3157 vehicles in pace Representing 53.2% of the total vehicles 27.6 mph· Vehicles > 65 MPH

Average Speed

0 0.0%

MassDOT Highway Division SPEED SUMMARY Tue 2/13/2018

File: SPDC1.prn Site Reference: 180040000798 City: BELLINGHAM Site ID: 00000000101 Location: ROUTE 126 SOUTH OF MAPLE ST. County: SPEED NB&SB Direction: NORTH Lane: 1 86+ Tota TIME 01:00 02:00 03:00 04:00 05:00 06:00 07:00 08:00 09:00 10:00 11:00 12:00 13:00 14:00 15:00 16:00 17:00 18:00 19:00 20:00 21:00 22:00 23:00 24:00 1081 1100 2114 2822 1947 DAY TOTAL 0.0% 0.0% 0.0% 0.0% 0.0% 0.0% 100% 11.4% 11.6% 22.2% 29.6% 20.4% 4.4% 0.4% 0.0% 0.0% PERCENTS

Statistical Information...

15th Percentile Speed 20.6 mph

Median Speed 29.8 mph

10 MPH Pace Speed 24 mph to 34 mph 4936 vehicles in pace Representing 51.7% of the total vehicles 85th Percentile Speed 36.6 mph

Average Speed 28.3 mph

Vehicles > 65 MPH 0.0%

#### MassDOT Highway Division SPEED SUMMARY Wed 2/14/2018

File: SPDC1.prn Site Reference: 180040000798 City: BELLINGHAM Site ID: 00000000101 County: SPEED NB&SB Location: ROUTE 126 SOUTH OF MAPLE ST. Direction: NORTH Lane: 1 86+ Tota TIME 01:00 02:00 03:00 04:00 05:00 06:00 07:00 08:00 09:00 10:00 11:00 12:00 13:00 14:00 15:00 16:00 17:00 . 96 18:00 19:00 20:00 21:00 22:00 23:00 24:00 991 2055 3068 1923 DAY TOTAL 0.0% 0.0% 0.0% 0.0% 0.0% 0.0% 0.0% 0.0% 100% 4.5% 10.9% 10.4% 21.5% 32.1% 20.1% 0.5% PERCENTS

Statistical Information ...

15th Percentile Speed 21.0 mph

Median Speed 30.2 mph

10 MPH Pace Speed 24 mph to 34 mph 5123 vehicles in pace Representing 53.4% of the total vehicles 85th Percentile Speed 36.6 mph

Average Speed 28.6 mph

Vehicles > 65 MPH 0.0%

#### MassDOT Highway Division SPEED SUMMARY Thu 2/15/2018

						×.										61 (R
Site Referen Site ID: 00 Location: R	0000000 OUTE 12	0101		MAPLE	ST.	1)				City:	SPDC1. BELLIN : SPEE		в			
Direction: 1 Lane: 1	NORTH							۵) ۵/								
TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
												24	2	-		
01:00	0	0	1	8	13	5	1	0	0	0	0	0	0	0	0	28
02:00	0	0	1	4	6	3	3 0	0	0	0	0	0	0	0	0	17
03:00	0	0	0	4	3	3 6 6	0	1 0	0	0	0	0	0	0	0	14
04:00	0	. 0	1	7	9		2		0	0	0	0	0	0	0	25
05:00	0	0	3	14	32	21	7	0	0	0	0	0	0	0	0	77
06:00	3	10	29	82	116	36	6	1	0	0	0	0	0	0	2	285
07:00	116	83	133	176	120	28	3		0	0	0	0	0	0	0	659
08:00	166	84	139	116	81	16	1	0	0	0	0	0.	0	0	0	603
09:00	190	128	140	130	54	2	2	1	0	0	0	0	0	0	0	647
						102		 2		0		0	0	0	2	2355
DAY TOTAL PERCENTS	475 20.28	305 13.0%	447 19.0%	541 23.0%	434 18.5%	123 5.2%	25 1.0%	3 0.1%	0 80.0	0.0%	0 0.0%	0.0%	0.0%	0.0%	0.0%	100%
Statistical	Inform	nation	• • •													с <u>з</u>
15th P	ercenti 14.1 n		eed								8	5th Pe	ercenti 36.7		ed	

Median Speed 28.5 mph Average Speed 26.5 mph Vehicles > 65 MPH 2 0.1%

10 MPH Pace Speed 24 mph to 34 mph 988 vehicles in pace Representing 41.9% of the total vehicles

#### MassDOT Highway Division SPEED SUMMARY Mon 2/12/2018

Site Refere Site ID: 00 Location: P Direction: Lane: 2	00000000 ROUTE 12	101		APLE S	ST.	57	-A. (	SB		City:	SPDC1. BELLIN : SPEE		В			
TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
12:00	4	19	176	232		2	0	0	0	0	0	0	0	0	0	491
13:00	23	31	191	259	44		0	0	0	0	0	0	0	0	0	550
14:00	4	13	187	244	58	2 2 4	0	0	0	. 0	0	0	0	0	0	508
15:00	17	66	250	246	50	4	0	0	0	0	0	0	0	0	0	633
16:00	5	29	297	341	88	1	0	0	1	0	0	0	. 0	0	0	762
17:00	62	54	249	300	50	1 2 1	1	0	0	0	. 0	0	0	0	0	718
18:00	180	97	213	162	25	1	0	1	0	0	0	0	0	0	0	679
19:00	21	32	237	260	46	1	0	0	0	0	0	. 0	0	0	0	597
20:00	2	1	133	166	44	1 1 3	1	0	0	0	0	0	0	0	0	348
21:00	2 3	.9	43	132	37		0	0	0	. 0	0	0	0	0	0	226
22:00	3	2	16	50	44	7	0	0	0	0	0	0	0	0	0	122
23:00	0	0	6	58	23	5	0	0	0	0	0	0	0	0	0	92
24:00	0	0	5	25	32	6	1	0	0	0	0	0	0	0	0	69
DAY TOTAL	323	353	2003	2475	599	37	3	1	1	0	0	0	0	0	0	5795
PERCENTS	5.6%	6.1%	34.6%	42.8%	10.3%	0.6%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information...

Median Speed 29.5 mph

15th Percentile Speed 24.5 mph 85th Percentile Speed 33.5 mph

Average Speed 28.5 mph

10 MPH Pace Speed 24 mph to 34 mph 4478 vehicles in pace Representing 77.2% of the total vehicles Vehicles > 65 MPH 0 0.0%

#### MassDOT Highway Division SPEED SUMMARY Tue 2/13/2018

File: SPDC1.prn Site Reference: 180040000798 Site ID: 00000000101 City: BELLINGHAM County: SPEED NB&SB Location: ROUTE 126 SOUTH OF MAPLE ST. Direction: SOUTH Lane: 2 86+ Tota TIME 01:00 02:00 03:00 04:00 05:00 06:00 07:00 08:00 09:00 10:00 11:00 12:00 13:00 14:00 Ω 15:00 16:00 17:00 18:00 19:00 20:00 21:00 22:00 23:00 24:00 397 2823 4260 1153 0 8885 DAY TOTAL 0.0% 0.0% 0.0% 0.0% 100% 0.8% 0.1% 0.0% 0.0% 0.0% 0.0% PERCENTS 1.9% 4.5% 31.8% 48.0% 12.9%

Statistical Information ...

15th Percentile Speed 25.4 mph

Median Speed 30.3 mph

10 MPH Pace Speed 24 mph to 34 mph 7083 vehicles in pace Representing 79.7% of the total vehicles 85th Percentile Speed 33.9 mph

Average Speed 29.8 mph

Vehicles > 65 MPH 0.0%

#### MassDOT Highway Division SPEED SUMMARY Wed 2/14/2018

File: SPDC1.prn Site Reference: 180040000798 Site ID: 00000000101 City: BELLINGHAM Location: ROUTE 126 SOUTH OF MAPLE ST. County: SPEED NB&SB Direction: SOUTH Lane: 2 86+ Tota TIME 01:00 02:00 03:00 04:00 05:00 06:00 07:00 08:00 09:00 10:00 11:00 12:00 .0 13:00 14:00 15:00 16:00 17:00 18:00 . 19:00 20:00 21:00 22:00 23:00 24:00 \_\_\_\_\_ 0 8855 534 2816 4125 1118 DAY TOTAL PERCENTS 1.9%

Statistical Information ...

15th Percentile Speed 25.1 mph

Median Speed 30.1 mph

10 MPH Pace Speed 24 mph to 34 mph 6941 vehicles in pace Representing 78.3% of the total vehicles 85th Percentile Speed 33.9 mph

Average Speed 29.6 mph

Vehicles > 65 MPH 0.0%

#### MassDOT Highway Division SPEED SUMMARY Thu 2/15/2018

Site Referent Site ID: 000 Location: Re Direction: Stane: 2	0000000 OUTE 12	101		MAPLE :	ST.					City:	SPDC1. BELLIN : SPEE		В	- - -		
TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
01:00	0	0	2	14	15	3	0	0	0	0	0	0	0	0	0	34
02:00	õ	Ő	3	8	7	0	0	0	0	0	0	0	0	0	0	18
03:00	Ö	õ	1	4	5	0	0	0	0	0	0	0	0	0	0	10
04:00	0	Ō	4	5	9	1	0	. 0	0	0	0	0	0	0	0	19
05:00	0	1	5	17	14	1	0	0	0	0	0	0	0	0	0	38
06:00	5	9	16	58	28	1 4 5	0 0	0	0	0	0	0	0	0	0	120
07:00	0	9	62	145	58	5	0	0	0	0	0	0	0	0	0	279
08:00	18	43	191	251	47	0 1	0	0	0	0	0	0	0	0	0	550
09:00	28	44	249	217	34	1	0	0	0	0	0	0	0	0	0	573
DAY TOTAL	 51	106	533	719	217	15	0	0	0	0	0	 0	0	0		1641
PERCENTS	3.2%			43.8%		0.9%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100%
Statistical	Inform	ation	•••													

15th Percentile Speed 24.8 mph

Median Speed 29.9 mph

10 MPH Pace Speed 24 mph to 34 mph 1252 vehicles in pace Representing 76.2% of the total vehicles 85th Percentile Speed 33.9 mph

Average Speed 29.3 mph

Vehicles > 65 MPH 0 0.0%

#### MassDOT Highway Division SPEED SUMMARY Mon 2/12/2018

Site Referer Site ID: 000 Location: MA Direction: H Lane: 1	0000000 APLE ST	203		F RTE.	126	57	A . 2	EB.		File: City: County	BELLIN	IGHAM	В			
TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
12:00	16	1	35	60	132	37	7	7	0	0	0	0	0	0	0	295
13:00	55	. 5	4	61	135	60	0	1	0	0	0	0	0	0	0	321
14:00	9	1	21	37	115	85	16	1 0	0	0	0	0	0	0	0	284
15:00	25	5	17	61	170	74	16	0	0	0	0	0	0	0	0	368
16:00	38	0	1	49	188	117	21	3	0	0	0	0	0	0	0	417
17:00	35	5	7	75	151	119	28	3 6 3	0	0	0	0	0	0	0	426
18:00	58	9	2	86	226	106	13	3	0	0	0	0	0	0	0	503
19:00	36	1	5	46	226	75	7	1	0	0	0	0	0	0	0	397
20:00	. 1	0	5	28	108	28	17	2	0	0	0	0	0	0	0	189
21:00		1	4	32	60	34	9	5	0	0	0	0	0	0	0	145
22:00	5 2	0	0	16	28	32	1	0	0	0	0	0	0	0	0	82
23:00		0	0	8	17	19	15	0 5	0	0	0	0	0	0	0	61
24:00	0	0	0	Ş	15	8	6	5	0	. 0	0	0	0	0	0	36
DAY TOTAL	280	28	101	561	1571	794	156	33	0	0	0	0	0	0	0	3524
PERCENTS	8.0%	0.8%	2.9%	16.0%	44.5%	22.5%	4.4%	0.9%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 30.1 mph

Median Speed 36.5 mph

10 MPH Pace Speed 34 mph to 44 mph 2365 vehicles in pace Representing 67.1% of the total vehicles

#### 85th Percentile Speed 41.9 mph

Average Speed 34.9 mph

Vehicles > 65 MPH 0 0.0%

#### MassDOT Highway Division SPEED SUMMARY Tue 2/13/2018

File: SPDC2.prn Site Reference: 180040000668 City: BELLINGHAM Site ID: 00000000203 County: SPEED EB&WB Location: MAPLE STREET EAST OF RTE. 126 Direction: EAST Lane: 1 86+ Tota TIME 01:00 02:00 03:00 04:00 05:00 06:00 07:00 08:00 09:00 10:00 11:00 12:00 13:00 14:00 15:00 16:00 17:00 18:00 19:00 20:00 21:00 22:00 23:00 24:00 808 2251 1366 0 5606 DAY TOTAL 0.0% 0.0% 0.0% 0.0% 0.0% 0.0% 0.0% 100% 2.1% 14.5% 40.1% 24.3% 5.3% 0.5% PERCENTS 12.5% 0.7% Statistical Information ...

15th Percentile Speed 28.7 mph

Median Speed 36.6 mph

10 MPH Pace Speed 34 mph to 44 mph 3617 vehicles in pace Representing 64.5% of the total vehicles 85th Percentile Speed 42.1 mph

Average Speed 34.0 mph

Vehicles > 65 MPH 0.0%

#### MassDOT Highway Division SPEED SUMMARY Wed 2/14/2018

File: SPDC2.prn Site Reference: 180040000668 City: BELLINGHAM Site ID: 00000000203 County: SPEED EB&WB Location: MAPLE STREET EAST OF RTE. 126 Direction: EAST Lane: 1 86+ Tota TIME 01:00 02:00 03:00 04:00 05:00 06:00 07:00 08:00 09:00 10:00 11:00 12:00 13:00 14:00 15:00 16:00 17:00 18:00 19:00 20:00 21:00 1. 22:00 23:00 24:00 0 5663 908 2360 1396 DAY TOTAL 0.0% 100% 4.9% 0.0% 0.0% 0.0% 0.0% 0.0% 0.0% 1.18 16.18 41.78 24.68 0.3% PERCENTS 0.8% 10.5%

Statistical Information ...

15th Percentile Speed 29.9 mph

Median Speed 36.6 mph

10 MPH Pace Speed 34 mph to 44 mph 3756 vehicles in pace Representing 66.3% of the total vehicles 85th Percentile Speed 42.1 mph

Average Speed 34.5 mph

Vehicles > 65 MPH 0.0%

#### MassDOT Highway Division SPEED SUMMARY Thu 2/15/2018

Site Refere Site ID: 00 Location: M Direction: Lane: 1	00000000 APLE ST	203		F RTE.	126					File: City: County	BELLIN	GHAM	B			
TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
01:00	0	0	0	6	6	2	7	4	0	· 0	0	0	0	0	0	25
02:00	0	0	0	0	6 2	2 0	. 6	0	0	0	0	0	0	. 0	0	8
03:00	0	5	0	0	0	0	0	0	0	0	0	0	. 0	0	0	. 5
04:00	0	4	0	0	12	5	0	0	0	0	0	. 0	0	0	0	21
05:00	0	0	0	12	2	25	0	0	0	0	0	0	0	0	0	39
06:00	8	0	9	35	27	28	18	0	0	0	0	0	0	0	0	125
07:00	78	0 3	4	57	91	54	36	0	. 0	0	0	0	0.	0	0	320
08:00	97	3	1	73	153	96	42	0 1	0	0	0	0	0	· 0	0	465
09:00	71	0	25	80	132	64	28	1	0	1	0	0	0	0	0	402
DAY TOTAL	254		39	263	425	274	137	5	0	1	0	0	0	 0	0	1410
PERCENTS	18.1%	0.9%			30.1%		9.7%	0.3%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100%
Statistical	L Inform	nation.	••									5 8 8				
1546 T	Donaonti	le Cre									. 6	85th Pe	ercenti	le Spe	ed	

15th Percentile Speed 15.8 mph

Median Speed 35.6 mph

10 MPH Pace Speed 34 mph to 44 mph 699 vehicles in pace Representing 49.5% of the total vehicles 85th Percentile Speed 42.8 mph

Average Speed 32.3 mph

Vehicles > 65 MPH 0 0.0%

# MassDOT Highway Division SPEED SUMMARY Mon 2/12/2018

Site Refere Site ID: 00 Location: M Direction: Lane: 2	00000000 APLE ST	203		F RTE.	126					File: City: County	BELLIN	GHAM	В	5		
		Га 101 г.	2.2	4 2 7			10	- 4	50	64	60	74	70	85	86+	Tota
TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	80	00+	IOLA
	0.6	~	10	20	1 6 0	60	10	1	0	0	0	0	0	0	0	319
12:00	26	0	16	38	168	60	10	1	1	0	0	0	2	0	0	320
13:00	61	11	3	57	104	79	1	1 3	T	0	0	0	0	0	. 0	321
14:00	34	0	27	53	129	59	16	3	1	0	3	0	0	0	6	395
15:00	97	1	10	71	98	98	7	3	T	0	3	0	0	0	0	536
16:00	153	0	- 4	40	207	86	26	16	0	0	0	0	0	4	0	493
17:00	121	0	4	56	162	125	18	3	0	0	0	4	0	0	0	
18:00	171	30	. 2	102	182	75	3	6	0	0	0	0	0	0	0	571
19:00	125	1	. 18	78	162	56	2	0	0	0	0	0	0	0	0	442
20:00	29	0	6	25	94	40		0	1	0	0	0	0	0	. 0	195
21:00	3	1	5	62	59	25	6	0	0	0	0	0	0	0	0	161
22:00	4	0	3	17	18	39	12	0	0	0	0	0	0	0	0	-93
23:00	3	0	0	13	39	9	11	0	0	0	0	. 0	0	0	0	75
24:00	3	0	0	5	17	9	10	0	0	0	0	0	0	0	0	44
	. <u>.</u>															
DAY TOTAL	830	44	98	617	1439	760	122	33	3	0	3	4	2	4	6	3965
PERCENTS	21.0%	1.2%	2.5%	15.6%	36.3%	19.2%	3.1%	0.8%	0.0%	0.0%	0.0%	0.1%	0.0%	0.1%	0.1%	100%

Statistical Information ...

15th Percentile Speed 13.6 mph

Median Speed 35.4 mph

10 MPH Pace Speed 34 mph to 44 mph 2199 vehicles in pace Representing 55.4% of the total vehicles 85th Percentile Speed 41.3 mph

Average Speed 31.2 mph

Vehicles > 65 MPH 19 0.5%

#### MassDOT Highway Division SPEED SUMMARY Tue 2/13/2018

Site Referen Site ID: 000 Location: MA	0000000 APLE STI	203		F RTE.	126					File: City: County	BELLIN	GHAM	В			
Direction: W Lane: 2	1001															
TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
01:00	0	0	0	8	8	9	. 0	0	0	0	0	0	0	0	0	25
02:00	0	0	0	5	4	0	0	0	0	0	0	0	0	0	. 0	9
03:00	0	0	5	4	5	5	0	0	0	. 0	0	0	0	0	0	19
04:00	0	0	0	2	2	6	3	0	0	0	0	0	0	0	0	13
05:00	0	0	0	1	. 12	6	7	3	0	0	0	0	0	0	0	29
06:00	2	0	6	11	44	27	25	1	0	0	0	0	0	0	0	116
07:00	23	0	. 4	34	129	100	18	3	1	1	0	0	0	0		313
08:00	54	0	3	39	190	137	18	0	0	0	0	0	0	0	0	441 390
09:00	54	3	11	61	148	87	25	1	0	0	0	0	0	0	0	255
10:00	38	0	7	52	104	37	13	0	0	0	0	0	0	4	0	
11:00	17	0	4	22	121	66	22	0	0	0	0	0	0	0		252
12:00	20	0	4	26	153	61	20	0	0	3	0	0	0	0	0	287 332
13:00	50	0	8	25	193	46	7	3	0	0	0	0	0	0	0	332
14:00	42	0	2	61	126	53	15	0	0	. 0	6	0	0	0	0	•
15:00	127	4	8	50	100	86	19	0	0	0	0	0	0	0	0	394
16:00	146	6	4	26	162	124	9	0	3	0	1	0	0	0	0	481
17:00	124	1	8	47	151	153	31	9	7	0	0	0	0	0	0	531
18:00	126	2	1	59	196	84	20	14	0	0	0	0	11	0	3	516
19:00	103	0	26	88	180	37	11	0	4	0	0	0	0	0	1	450
20:00	24	0	0	64	126	20	8	0	0	0	0	0	0	0	0	242 229
21:00	36	0	14	42	90	36	11	0	0	0	0	0	0	0	0	158
22:00	10	0	1	34	70	33	10	0	0	0	0	0	0	0	100	158
23:00	4	0	0	5	43	15	1	1	0	0	0	0	0	0	0	40
24:00	0	0	0	13	10	10	7	0	0	0	0	0	0	0	0	40
DAY TOTAL	1000	 16	116	779		1238	300	. 35	15	4	7	0		4	4	5896
PERCENTS	17.0%	0.3%	2.0%	13.3%	40.2%	21.0%	5.1%	0.6%	0.3%	0.0%	0.1%	0.0%	0.1%	0.0%	0.0%	100%

Statistical Information...

15th Percentile Speed 16.8 mph

Median Speed 36.2 mph

10 MPH Pace Speed 34 mph to 44 mph 3605 vehicles in pace Representing 61.1% of the total vehicles 85th Percentile Speed 42.0 mph

#### Average Speed 32.9 mph

Vehicles > 65 MPH 26 0.4%

#### MassDOT Highway Division SPEED SUMMARY Wed 2/14/2018

File: SPDC2.prn Site Reference: 180040000668 City: BELLINGHAM Site ID: 00000000203 County: SPEED EB&WB Location: MAPLE STREET EAST OF RTE. 126 Direction: WEST Lane: 2 86+ Tota TIME \_\_\_\_ 01:00 . 0 02:00 03:00 04:00 05:00 06:00 07:00 08:00 09:00 10:00 48. 11:00 12:00 13:00 . 1 14:00 15:00 16:00 17:00 .0 . 0 18:00 19:00 20:00 - 1 21:00 22:00 23:00 24:00 \_\_\_\_ 787 2282 1461 1 6047 DAY TOTAL 2.7% 13.1% 37.7% 24.1% 5.1% 0.8% 0.0% 0.0% 0.0% 0.0% 0.0% 0.0% 100% PERCENTS 15.1% 1.4%

Statistical Information ...

15th Percentile Speed 19.0 mph

Median Speed 36.4 mph

10 MPH Pace Speed 34 mph to 44 mph 3743 vehicles in pace Representing 61.8% of the total vehicles 85th Percentile Speed 42.2 mph

Average Speed 33.2 mph

Vehicles > 65 MPH 0.1%

#### MassDOT Highway Division SPEED SUMMARY Thu 2/15/2018

Site Refere Site ID: 00 Location: M Direction: Lane: 2	0000000 APLE ST	203		F RTE.	126					File: City: County	BELLIN	GHAM	B			
TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
		·	0	12	 o	5	 · 5	1	. 0	0	0	0	0	0	0	25
01:00 02:00	0	0	0		2	7	· 5 0	Ō	Ő	õ	Ő	õ	õ	õ	Ő	13
02:00	0	2	0	0	0	2	Ő	Ő	0	0	0	õ	õ	Ö	õ	4
04:00	0	0	1	3	2		7	1	ő	õ	õ	õ	0	Ō	0	15
05:00	0	0	0	1	19	1 9	2	1		õ	0	0	0	0	0	32
06:00	0	Ő	12	37	36	19	10	1 3	0	0	0	0	0	. 0	0	117
07:00	10	õ	6		112	107	11	0	4	0	0	0	0	0	0	300
08:00	35	1	3	59	185	117	29	0 3	0	0	0	0	0	0	2	431
09:00	65	1	9		128	97	19	3	0	0	0	1	0	0	0	374
DAY TOTAL	110	 4	31	219	484	364	83	9	4	0	0	1	0	. 0	2	1311
PERCENTS	8.4%	0.4%					6.3%	0.6%	0.3%	0.0%	0.0%	0.0%	0.0%	0.0%	0.1%	100%

Statistical Information ...

15th Percentile Speed 30.2 mph

Median Speed 37.0 mph

10 MPH Pace Speed 34 mph to 44 mph 848 vehicles in pace Representing 64.6% of the total vehicles 85th Percentile Speed 42.7 mph

Average Speed 35.4 mph

Vehicles > 65 MPH 3 0.2%

#### MassDOT Highway Division SPEED SUMMARY Mon 2/12/2018

Site Refere Site ID: 00 Location: 1 Direction: Lane: 1	00000000 ROUTE 12	302		IAPLE :	ST.	57	A . 2	SB	8	City:	SPD302 BELLIN 7: SPEE	IGHAM	an a		5) 2) 5)	
TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
12:00	202			21	191	145	1 /		0		0		0			574
12:00	186	1 13	0 32	65	261	145	14 30	0 0	0	0	0	0	0	· 0	0	574 694
14:00	217	13	13	43	196	179	21	0	0	0	0	0	0	0	0	677
15:00	192	12	20	34	200	198	45	2	0	0	0	0	0	0	0	704
16:00	220	16	20	54 64	175	198	43 54	3 0	0	0	0	0	0	0	0	731
17:00	241	2	22	66	255	148	25	0	0	1	0	0	0	0	0	740
18:00	241	8	11	111	233	97	12		0	0	0	0	0	0	0	703
19:00	198	8	4	60	222	136	37	0	0	0	0	0	0	0	0	665
20:00	177	2	6	10	159	167	14	0 0 2	0	0	ő	Ő	Ő	0	0	537
20:00	89	0	3	20	83	228	43	0	0	3	0	Ő	0	0	Ő	469
22:00	38	3 3	0		106	123	61	0	Ő	0	Ő	Ő	õ	Ő	Ő	340
23:00	6	0	0	- 7	27	156	43	4	õ	õ	0	ŏ	Ő	Ő	Ő	243
24:00	4	Ő	Ő	1	20	66	47	8	Ő	Ő	0	Ő	0	0	0	146
DAY TOTAL	 1996	73	113	511	2133	1930	446	17	0	4	0	0	0	0	0	7223
PERCENTS	27.7%	1.18	1.6%	7.1%			6.1%	0.2%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	100%

Statistical Information ...

15th Percentile Speed 10.3 mph

Median Speed 36.2 mph

10 MPH Pace Speed 34 mph to 44 mph 4063 vehicles in pace Representing 56.2% of the total vehicles 85th Percentile Speed 42.4 mph

Average Speed 30.4 mph

Vehicles > 65 MPH 0 0.0%

#### MassDOT Highway Division SPEED SUMMARY Tue 2/13/2018

Site Refere Site ID: 00 Location: R Direction: Lane: 1	0000000 OUTE 12	302		APLE S	ST.	28 1				City:	SPD302 BELLIN : SPEE	GHAM				
TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
01:00	1	0	6	1	12	27	33	13	1	0	0	0	. 0	0	0	94
02:00	0	0	0	3	0	19	12	3	0	0	0	0	0	0	0	37 46
03:00	0	0	0	0	8	35	3	0	0	0	0	0	0	0 0	0	46
04:00	0	0	9	0	2	21	38	1	1	0	0	0	0	0	0	205
05:00	24	0	10	10	26	64	55	16	0	0	0	0	0	9	0	465
06:00	90	10	6	16	68	196	70	0	. 0	553	0	0	0	0	. 0	622
07:00	389	13	2	31	64	63	47	13	0	0	0	0	0	0	0	636
08:00	254	8	7	20	120	138	76	9	4	0	. 0	0	0	0	0	688
09:00	239	5	5	40	157	190	49	3	•	2	3	4	0	0	1	650
10:00	158	4	23	40	178	180	52	5	0		3	4	2	3	5	760
11:00	90	1	9	34	238	295	66	8	1	1	4	4	2	3	1	781
12:00	64	1	7	51	251	288	87	14	1	3 5	4	4	- 3	1	0	843
13:00	79	1	5	58	300	300	74	11	0	5	3	0	0	1	1	862
14:00	74	6	12	76	282	345	60	2	0	0	0	1	0	0	3	962
15:00	70	10	30	84	377	321	61	5	°.	3	0	1	0	2	4	1011
16:00	109	4	26	150	434	229	45	2	2	3	1	4	0	0	4	980
17:00	128	5.	26	97	379	269	54	7		1	2	4	0	0	. 0	1034
18:00	136	2	- 13	106	410	313	41	6	1	1	2	1	1	1	5	895
19:00	94	2	10	84	372	268	51	5 6.	•	1	2	0	0	0	0	730
20:00	31	0	2	39	304	299	46	_	0	0	2	1	0	1	1	580
21:00	28	1	6	26	181	251	78	5	2	1	0	0	0	2	0 0	352
22:00	10	2	1	9	116	148	56	11- <del>34</del> .00	2 5	0	0	0	0	0	0	181
23:00	4	1	1	- 4	34	.87	36	9 6	1	1	1	0	0	0 0	0	139
24:00	2	0	1	5	28	66	28	0	Т	1	Т	0	U	0	0	100
DAY TOTAL	2074	76	217	984	4341	4412	1218	154	23	19	26	20	8	23		13625
PERCENTS	15.3%	0.6%	1.6%	7.3%		32.4%	9.0%	1.2%	0.1%	0.1%	0.1%	0.1%	0.0%	0.1%	0.2%	100%
Statistical	l Inform	nation.	• •													

15th Percentile Speed

18.7 mph

Median Speed 38.0 mph

10 MPH Pace Speed 34 mph to 44 mph 8753 vehicles in pace Representing 64.2% of the total vehicles 85th Percentile Speed 43.4 mph

Average Speed 34.8 mph

Vehicles > 65 MPH 107 0.8%

# MassDOT Highway Division SPEED SUMMARY Wed 2/14/2018

Site Refere Site ID: 00 Location: P Direction: Lane: 1	00000000 ROUTE 12	302		APLE S	т.	*				File: City: County	BELLIN	GHAM				
TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
01:00	0	0	0	3	16	28	16	1	1	. 0	0	2	0	0	0	67
02:00	0	1	0	1	9	8	13	3	1	0	0	0	0	0	0	36
03:00	0	0	0	0	7	14	1	0	0	0	0	0	0	0	0	22
04:00	0	0	1	1	8	18	22	6	2	0	0	0	0	0	0	58
05:00	0	0	1	6	21	42	50	23	1	0	0	0	0	0	0	144
06:00	47	0	1	13	67	163	101	15	. 6	0	2	2	2	5	4	428
07:00	84	6	13	41	222	370	101	20	1	2	2	2	2	0	3	869
08:00	121	7	11	67	283	338	88	21	1	. 1	2	0	0	1	1	942
09:00	89	3	16	71	313	331	82	3	4	0	4	0	0	2	2	920
10:00	93	1	3	68	248	267	89	16	3	4	4	7	2	2	3	810
11:00	97	2	17	65	214	278	72	12	6	6	8	2	5	1	1	786
12:00	115	7	12	24	213	310	72	12	5	0	10	1	1	. 3	. 1	786
13:00	136	2	8	34	199	264	82	7	5	7	10	4	0	2	2	762
14:00	171	0	4	43	247	216	55	8	3	2	4	2	. 1	0	4	760
15:00	254	5	32	60	155	163	16	0	0	0	0	0	0	0	. 0	685
16:00	246	4	15	33	210	150	20	1	0	0	0	0	0	0	4	683
17:00	231	23	21	67	201	176	25	2	0	0	0	0	0	0	0	746
18:00	211	6	27	75	223	154	28	1	3	4	5	2	4	1	0	744
19:00	157	2	16	102	265	169	42	7	1	4	5	2	Ţ	0	1	774
20:00	82	0	7	59	191	232	48	10	4	4	8	5	3	0	0	653
21:00	49	2	8	32	158	192	44	8	0	3	1	10	4	0	0	511
22:00	40	2	2	28	106	152	67	14	. 1	0	6	0	0	1	2	421
23:00	11	0	0	6	84	110	36	8	0	4	5	0	5	0	0	269 176
24:00	4	3	0	2	27	77	51	6	0	6	0	0	0	-0	0	1/0
DAY TOTAL	2238	76	215	901	3687	4222	1221	204	48	47	76	41	30	18		13052
PERCENTS	17.2%	0.6%	1.7%	7.0%	28.3%	32.4%	9.4%	1.5%	0.3%	0.3%	0.5%	0.3%	0.2%	0.1%	0.2%	100%

Statistical Information...

15th Percentile Speed 16.6 mph

Median Speed 38.2 mph

10 MPH Pace Speed 34 mph to 44 mph 7909 vehicles in pace Representing 60.5% of the total vehicles

85th Percentile Speed 43.7 mph

Average Speed 34.7 mph

Vehicles > 65 MPH 193 1.5%

#### MassDOT Highway Division SPEED SUMMARY Thu 2/15/2018

Site Refere Site ID: 00 Location: F Direction: Lane: 1	00000000 ROUTE 12	302		IAPLE S	ST.			(94)		File: City: County	BELLIN	GHAM				<sup>р</sup> а. 
TIME	19	24	29	34	39	44	49	54	59	64	69	74	79	85	86+	Tota
01:00	0	1	0	2	13	36	26	7	0	0	0	0	0	0	0	85 48
02:00	0	0 0	0	0 0	14 5	20 20	11 5	2	1	0	0	0	0	0	0	40 30
03:00 04:00	10	0	8	1	6	24	19	. 0	ő	Ő	ő	Ő	Ő	õ	õ	69
05:00	18	0	3	ō	46	52	40	4	5	õ	0	0	0	0	0	168
06:00	153	4	0	15	68	112	78	0	0	0	0	0	0	0	0	430
07:00	342	8	4	11	86	121	38	8	0	3	0	0	0	3	0	624
08:00	265	32	26	45	113	146	24	0	0	. 0	0	0	0	0	0	651
09:00	255	19	16	33	188	109	24	0	- 0	0	1	0	0	0	0	645
DAY TOTAL	1043	64	57	107	539	 640	265	22	 6	3	 1	0	0	3	0	2750
PERCENTS	38.0%	2.4%	2,1%		19.6%		9.6%	0.8%	0.2%	0.1%	0.0%	0.0%	0.0%	0.1%	0.0%	100%
8		2														

Statistical Information...

15th Percentile Speed 7.5 mph

Median Speed 35.0 mph

10 MPH Pace Speed 34 mph to 44 mph 1179 vehicles in pace Representing 42.8% of the total vehicles 85th Percentile Speed 43.1 mph

Average Speed 27.9 mph

Vehicles > 65 MPH 4 0.1%

Appendix C: Crash Data Analysis

### Collision Diagram Look-up MassDOT 2011-2015 Crash Data

Index	Crash Number	Police Report ID	Crash Time	Crash Date Crash Location	Crash Severity	Manner of Collision	Vehicle Traveled Direction	Road Surface	Ambient Light Condition	Weather Bike/ Ped	Vehicle Action
1	2680767	11-6-AC	9:45 AM	2011-01-05 Hartford Ave / Maple St	Property damage only	Rear-end	V1:Eastbound / V2:Eastbound	Dry	Daylight	Clear	V1: Slowing or stopped in traffic / V2:Travelling straight ahead
2	2685541	11-60-AC	10:58 AM	2011-01-31 Hartford Avenue	Property damage only	Rear-end	V1:Westbound / V2:Westbound	Dry	Daylight	Clear	V1: Slowing or stopped in traffic / V2:Travelling straight ahead
3	2715374	11-189-AC	8:36 AM	2011-04-13 50 Feet E From Stallbroook School	Property damage only	Rear-end	V1:Westbound / V2:Westbound	Wet	Daylight	Rain	V1: Slowing or stopped in traffic / V2:Travelling straight ahead
4	2751067	11-329-AC	10:27 PM	2011-07-29 Hartford Avenue	Property damage only	Angle	V1:Eastbound / V2:Westbound	Wet	Dark - lighted road	w Clear	V1: Slowing or stopped in traffic / V2:Turning left
5	2949458	12-99-AC	6:35 PM	2012-03-07 Maple St	Property damage only	Rear-end	V1:Northbound / V2:Northbound	Dry	Dark - lighted road	w Clear	V1: Slowing or stopped in traffic / V2:Travelling straight ahead
6	3047971	12-140-AC	2:47 PM	2012-04-12 Hartford Ave	Property damage only	Rear-end	V1:Northbound / V2:Northbound	Wet	Daylight	Rain	V1: Travelling straight ahead / V2:Travelling straight ahead
7	3245098	12-365-AC	8:39 AM	2012-08-20 Hartford Ave / Maple St	Property damage only	Rear-end	V1:Westbound / V2:Westbound	Dry	Daylight	Cloudy	V1: Slowing or stopped in traffic / V2:Travelling straight ahead
8	3288084	12-496-AC	6:52 PM	2012-11-05 Hartford Ave	Property damage only	Rear-end	V1:Northbound / V2:Northbound	Dry	Dark - lighted road	w Clear	V1: Slowing or stopped in traffic / V2:Travelling straight ahead
9	3319090	12-555-AC	4:21 PM	2012-12-12 Hartford Ave	Property damage only	Rear-end	V1:Northbound / V2:Northbound	Dry	Dusk	Clear	V1: Slowing or stopped in traffic / V2:Travelling straight ahead
10	3381815	13-126-AC	4:55 PM	2013-04-01 Hartford Ave	Property damage only	Rear-end	V1:Eastbound / V2:Eastbound	Wet	Daylight	Rain	V1: Slowing or stopped in traffic / V2:Travelling straight ahead
11	3434123	13-233-AC	7:25 AM	2013-06-03 Hartford Ave / Maple St	Property damage only	Angle	V1:Northbound / V2:Eastbound	Wet	Daylight	Rain	V1: Travelling straight ahead / V2:Turning left
12	3471521	13-247-AC	4:11 PM	2013-06-10 200 Feet E From Intersection 318 Hartfor	n Property damage only	Rear-end	V1:Eastbound / V2:Westbound / V3:	:EWet	Daylight	Rain	V1: Slowing or stopped in traffic / V2:Slowing or stopped in traffic / V3:Trave
13	3595354	13-429-AC	11:38 AM	2013-09-21 Hartford Ave	Property damage only	Rear-end	V1:Eastbound / V2:Eastbound	Dry	Daylight	Clear	V1: Travelling straight ahead / V2:Slowing or stopped in traffic
14	3710099	13-594-AC	5:46 PM	2013-12-26 Hartford Ave	Non-fatal injury	Rear-end	V1:Westbound / V2:Westbound	Snow/Ice	Dark - lighted road	w Snow	V1: Slowing or stopped in traffic / V2:Travelling straight ahead
15	3750359	14-110-AC	10:50 AM	2014-02-27 Hartford Ave	Property damage only	Head-on	V1:Eastbound / V2:Northbound	Dry	Daylight	Clear	V1: Travelling straight ahead / V2:Travelling straight ahead
16	3791588	14-177-AC	3:34 PM	2014-04-13 Hartford Ave	Property damage only	Rear-end	V1:Eastbound / V2:Eastbound	Dry	Daylight	Clear	V1: Slowing or stopped in traffic / V2:Slowing or stopped in traffic
17	3881497	14-309-AC	2:59 PM	2014-07-05 Hartford Ave	Non-fatal injury	Unknown	V1:Eastbound / V2:Eastbound	Dry	Daylight	Clear	V1: Travelling straight ahead / V2:Travelling straight ahead
18	4030834	15-201-AC	7:48 AM	2015-04-06 Hartford Avenue / Maple Street	Property damage only	Rear-end	V1:Westbound / V2:Westbound / V3	3:\Dry	Daylight	Clear	V1: Travelling straight ahead / V2:Travelling straight ahead / V3:Travelling s
19	4155506	15-648-AC	5:08 PM	2015-11-05 Hartford Ave / Maple St	Property damage only	Rear-end	V1:Eastbound / V2:Eastbound	Dry	Dark - lighted road	w Clear	V1: Travelling straight ahead / V2:Travelling straight ahead
20	4155507	15-649-AC	6:00 AM	2015-11-06 Hartford Ave	Non-fatal injury	Rear-end	V1:Eastbound / V2:Eastbound / V3:	E≀Wet	Daylight	Cloudy	V1: Slowing or stopped in traffic / V2:Slowing or stopped in traffic / V3:Slowi



# **INTERSECTION CRASH RATE WORKSHEET**

CITY/TOWN : Bellingham	ı		_	COUNT DAT	E : 2/12/2018	8 – 2/14/2018
DISTRICT : 3	UNSIGN	ALIZED :		SIGNA	LIZED :	X
		~ IN	ITERSECTION	DATA ~		
	 Route 126 (H					
MINOR STREET(S):	Maple Street					
	Driveway					
INTERSECTION DIAGRAM			Stay Brook	(B) 449 4		Peddingstone La
					2	ambet
APPROACH :	1	2	PEAK HOUR 3	4	5	Total Peak
DIRECTION :	NB	SB	WB	EB	•	Hourly Approach
PEAK HOURLY VOLUMES (AM/PM) :	695	675	240	8		Volume 1,618
"K" FACTOR:	0.090	INTERS	ECTION ADT APPROACH	. ,	L DAILY	17,972
TOTAL # OF CRASHES :	20	# OF YEARS :	5	CRASHES	GE # OF PER YEAR	4.00
CRASH RATE CALCU	LATION :	0.61	RATE =	( A* 1,0 ( V	000,000 ) * 365 )	
Comments :		-	signalized int			
Project Title & Date:	Hartford Aver	nue and Maple	e Street Inters	ection Redes	ign Study, Jul	ly 2018

Appendix D: Intersection Levels of Service

# Lanes and Geometrics 1: Hartford Ave (Rt. 126) & Maple Street

07/18/2018

	•	•	<b>†</b>	1	1	Ŧ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		4		<u> </u>	<u> </u>
Traffic Volume (vph)	130	180	655	40	65	430
Future Volume (vph)	130	180	655	40	65	430
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Satd. Flow (prot)	1625	0	1720	0	1616	1749
Flt Permitted	0.980	0	1720	0	0.156	1/49
		0	1700	0		1749
Satd. Flow (perm)	1625	0	1720	0	265	1/49
Right Turn on Red		No		No		
Satd. Flow (RTOR)	20		20			20
Link Speed (mph)	30		30			30
Link Distance (ft)	1130		962			231
Travel Time (s)	25.7		21.9			5.3
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Heavy Vehicles (%)	2%	2%	6%	5%	8%	5%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	323	0	724	0	68	448
Turn Type	Prot		NA		pm+pt	NA
Protected Phases	8		2		1	6
Permitted Phases			_		6	-
Detector Phase	8		2		1	6
Switch Phase	0		2			0
Minimum Initial (s)	6.0		15.0		6.0	15.0
Minimum Split (s)	11.0		20.0		11.0	20.0
Total Split (s)	20.0		50.0		20.0	70.0
Total Split (%)	22.2%		55.6%		22.2%	77.8%
Yellow Time (s)	4.0		4.0		4.0	4.0
All-Red Time (s)	1.0		1.0		1.0	1.0
Lost Time Adjust (s)	0.0		0.0		0.0	0.0
Total Lost Time (s)	5.0		5.0		5.0	5.0
Lead/Lag			Lag		Lead	
Lead-Lag Optimize?						
Recall Mode	None		Min		None	Min
Act Effct Green (s)	15.8		35.2		43.9	43.9
Actuated g/C Ratio	0.23		0.50		0.63	0.63
v/c Ratio	0.88		0.84		0.23	0.41
Control Delay	59.5		25.6		6.0	7.1
Queue Delay	0.0		0.0		0.0	0.0
Total Delay	59.5		25.6		6.0	7.1
LOS	59.5 E		25.0 C		0.0 A	
					А	A
Approach Delay	59.5		25.6			6.9
Approach LOS	E		С		^	A
Queue Length 50th (ft)	149		265		9	80
Queue Length 95th (ft)	#349		430		20	123
Internal Link Dist (ft)	1050		882			151
Turn Bay Length (ft)						
Base Capacity (vph)	365		1156		469	1543
Starvation Cap Reductn	0		0		0	0
Spillback Cap Reductn	0		0		0	0
Storage Cap Reductn	0		0		0	0

Existing AM

	1	•	Ť	~	1	Ļ	
Lane Group	- WBL	WBR	NBT	NBR	SBL	SBT	
Reduced v/c Ratio	0.88	WDIX	0.63	NBR	0.14	0.29	
Intersection Summary							
Area Type:	Other						
Cycle Length: 90							
Actuated Cycle Length: 7	0.2						
Natural Cycle: 70							
Control Type: Actuated-U	ncoordinated						
Maximum v/c Ratio: 0.88							
Intersection Signal Delay				Int	tersection	LOS: C	
Intersection Capacity Utili	zation 72.7%			IC	U Level o	f Service C	
Analysis Period (min) 15							
# 95th percentile volum	e exceeds ca	pacity, qu	eue may	be longer	•		
Queue shown is maxir	num after two	cycles.					

Splits and Phases: 1: Hartford Ave (Rt. 126) & Maple Street



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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्स	eî 👘	
Traffic Volume (vph)	10	15	20	815	490	10
Future Volume (vph)	10	15	20	815	490	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Satd. Flow (prot)	1643	0	0	1749	1746	0
Flt Permitted	0.980			0.999		
Satd. Flow (perm)	1643	0	0	1749	1746	0
Link Speed (mph)	30			30	30	
Link Distance (ft)	589			231	621	
Travel Time (s)	13.4			5.3	14.1	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	2%	0%	0%	5%	5%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	27	0	0	879	527	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	d					
Intersection Capacity Utiliz	zation 69.0%			IC	U Level o	of Service C
Analysis Period (min) 15						

`	٨	>	•	ŧ	1	1
		V	١		▼	•
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्भ	4Î	
Traffic Volume (veh/h)	10	15	20	815	490	10
Future Volume (Veh/h)	10	15	20	815	490	10
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	11	16	21	858	516	11
Pedestrians						
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (ft)				231		
pX, platoon unblocked	0.63			<u> </u>		
vC, conflicting volume	1422	522	527			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol						
vCu, unblocked vol	1375	522	527			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	011	0.2				
tF (s)	3.5	3.3	2.2			
p0 queue free %	89	97	98			
cM capacity (veh/h)	99	559	1050			
Direction, Lane #	EB 1 27	NB 1	SB 1 527			
Volume Left	27 11	879 21				
			0			
Volume Right	16	0	11			
cSH	193	1050	1700			
Volume to Capacity	0.14	0.02	0.31			
Queue Length 95th (ft)	12	2	0			
Control Delay (s)	26.7	0.5	0.0			
Lane LOS	D	А				
Approach Delay (s)	26.7	0.5	0.0			
Approach LOS	D					
Intersection Summary						
Average Delay			0.8			
Intersection Capacity Utilization	ation		69.0%	IC	CU Level d	of Service
Analysis Period (min)			15			
			10			

# Lanes and Geometrics 1: Hartford Ave (Rt. 126) & Maple Street

07/18/2018

Lane Group         WBL         WBR         NBT         NBR         SBL         SBT           Lane Configurations         Y		4	•	Ť	۲	1	ţ
Lane Configurations         Image: Configuration of the image: Configurating of the image: Configuration of the image: Configuration of t	Lane Group	WBI	WBR	NBT	NBR	SBL	SBT
Traffic Volume (vph) $80$ 90 $575$ 120       200 $655$ Ideal Flow (vphp)       1900       190       190       110       100 <td< td=""><td></td><td></td><td>TH DIC</td><td></td><td>HDR</td><td></td><td></td></td<>			TH DIC		HDR		
Future Volume (vph)         80         90         575         120         200         655           Ideal Flow (vphp)         1900         1900         1900         1900         1900           Satd. Flow (port)         1636         0         1759         0         128         1801           Right Turn on Red         No         No         No         Satd. Flow (RTOR)         1130         962         231           Link Speed (mph)         30         30         30         30         30           Link Distance (ft)         1130         962         231         723           Peak Hour Factor         0.91         0.91         0.91         0.91         0.91           Heavy Vehicles (%)         1%         0%         2%         2%         1%         2%           Shared Lane Traffic (%)            16         720         720           Turn Type         Prot         NA         pm+pt         NA         720         720           Turn Type         Prot         NA         16         720         720         720           Turn Type         Prot         NA         90         1.0         1.0         2.0 </td <td></td> <td></td> <td>90</td> <td></td> <td>120</td> <td></td> <td></td>			90		120		
Ideal Flow (vphp)         1900 <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>							
Satd. Flow (prot)         1636         0         1759         0         1728         1801           Flt Permitted         0.977         0.130         0.110         0.100         0.100         0.100         0.100         0.01         0.01         0.01         0.01         0.01         0.01         0.01         0.110         0.110         0.100         0.110         0.110         1.00         0.110         1.00         0.110         1.00         1.01         1.01         1.01         0.110         1.01         1.01         1.01         1.01         1.01         1.01         1.01         1.01         1.01         1.01         1.01         1.01         1.01         1.01         1.01         1.01         1.01         1.01         1.0	· · · ·						
Fit Permitted         0.977         0.130           Satd. Flow (perm)         1636         0         1759         0         236         1801           Right Turn on Red         No         No         No         Satd. Flow (RTOR)         1130         962         231           Link Speed (mph)         30         30         30         30         30           Link Distance (ft)         1130         962         231         720           Travel Time (s)         25.7         21.9         5.3         Confl. Peds. (#/hr)         2           Peak Hour Factor         0.91         0							
Satd. Flow (perm)         1636         0         1759         0         236         1801           Right Turn on Red         No         No         No         Satd. Flow (RTOR)           Link Speed (mph)         30         30         30         30           Link Distance (ft)         1130         962         231           Travel Time (s)         25.7         21.9         5.3           Conft. Peds. (#/hr)         2         Peak Hour Factor         0.91         Heavy Vehicles (%)         1%         0%         2%         %         Shared Lane Traffic (%)         2         1%         0         220         720         Turn Type         Prot         NA         pm+pt         NA           Permitted Phases         3         2         1         6         Switch Phase         10         1.0         1.0         1.0	4 7		0	1737	0		1001
Right Turn on Red         No         No           Satd. Flow (RTOR)         30         30         30           Link Speed (mph)         30         30         30           Link Distance (ft)         1130         962         231           Travel Time (s)         25.7         21.9         5.3           Confl. Peds. (#/hr)         2         2         2%           Peak Hour Factor         0.91         0.91         0.91         0.91         0.91         0.91           Heavy Vehicles (%)         1%         0%         2%         2%         1%         2%           Shared Lane Traffic (%)         Lane Group Flow (vph)         187         0         764         0         220         720           Turn Type         Prot         NA         pm+pt         NA           Protected Phases         3         2         1         6           Switch Phase         3         2         1         6           Minimum Initial (s)         6.0         15.0         6.0         15.0           Moinmum Split (s)         11.0         20.0         70.0         70.0           Total Split (%)         22.2%         75.6%         22.2%			0	1750	0		1001
Said. Flow (RTOR)         Jink Speed (mph)         30         30         30           Link Distance (ft)         1130         962         231           Travel Time (s)         25.7         21.9         5.3           Confl. Peds. (#/hr)         2         2         28           Peak Hour Factor         0.91         0.91         0.91         0.91         0.91           Heavy Vehicles (%)         1%         0%         2%         2%         1%         2%           Shared Lane Traffic (%)            20         720         720           Lane Group Flow (vph)         187         0         764         0         220         720           Turn Type         Prot         NA         pm+pt         NA           Protected Phases         3         2         1         6           Switch Phase         -         6         15.0         6.0         15.0           Minimun Initial (s)         6.0         15.0         6.0         15.0         10.0         10.0         10.0         10.0         10.0         10.0         10.0         10.0         10.0         10.0         10.0         10.0         10.0         10.0	4 7	1030		1/09		230	1001
Link Speed (mph)         30         30         30         30           Link Distance (ft)         1130         962         231           Travel Time (s)         25.7         21.9         5.3           Confl. Peds. (#/hr)         2         Peak Hour Factor         0.91         D.91         D.			INO		INO		
Link Distance (ft)         1130         962         231           Travel Time (s)         25.7         21.9         5.3           Confi. Peds. (#/hr)         2         2           Peak Hour Factor         0.91         0.91         0.91         0.91         0.91         0.91           Heavy Vehicles (%)         1%         0%         2%         2%         1%         2%           Shared Lane Traffic (%)         1%         0%         764         0         220         720           Turn Type         Prot         NA         pm+pt         NA           Protected Phases         3         2         1         6           Detector Phase         3         2         1         6           Switch Phase	, <i>,</i>	20		20			20
Travel Time (s)         25.7         21.9         5.3           Confl. Peds. (#/hr)         2         2           Peak Hour Factor         0.91         0.91         0.91         0.91         0.91         0.91           Heavy Vehicles (%)         1%         0%         2%         2%         1%         2%           Shared Lane Traffic (%)         Lane Group Flow (vph)         187         0         764         0         220         720           Turn Type         Prot         NA         pm+pt         NA           Protected Phases         3         2         1         6           Detector Phase         3         2         1         6           Switch Phase         Minimun Initial (s)         6.0         15.0         6.0         15.0           Minimun Initial (s)         6.0         15.0         6.0         15.0         10.0         1.0         20.0           Total Split (s)         20.0         50.0         20.0         70.0         77.8%           Yellow Time (s)         4.0         4.0         4.0         4.0         4.0           All-Red Time (s)         1.0         1.0         1.0         1.0         1.0         1.0 <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>							
Confl. Peds. $(\#/hr)$ 2           Peak Hour Factor         0.91         0.91         0.91         0.91         0.91         0.91         0.91           Heavy Vehicles (%)         1%         0%         2%         2%         1%         2%           Shared Lane Traffic (%)            2         1%         2%           Lane Group Flow (vph)         187         0         764         0         220         720           Turn Type         Prot         NA         pm+pt         NA           Protected Phases         3         2         1         6           Permitted Phases         6           6           Detector Phase         3         2         1         6           Switch Phase            70.0           Total Split (\$)         20.0         50.0         20.0         70.0           Total Split (\$)         20.0         50.0         20.0         70.0           Total Split (\$)         20.0         0.0         0.0         0.0           Lead Split (\$)         0.0         0.0         0.0         0.0           Lead/Lag	.,						
Peak Hour Factor         0.91         0.91         0.91         0.91         0.91         0.91         0.91           Heavy Vehicles (%)         1%         0%         2%         2%         1%         2%           Shared Lane Traffic (%)         1%         0%         2%         2%         1%         2%           Lane Group Flow (vph)         187         0         764         0         220         720           Turn Type         Prot         NA         pm+pt         NA           Protected Phases         3         2         1         6           Detector Phase         3         2         1         6           Switch Phase	• •	25.7		21.9			5.3
Heavy Vehicles (%)         1%         0%         2%         2%         1%         2%           Shared Lane Traffic (%)         Lane Group Flow (vph)         187         0         764         0         220         720           Turn Type         Prot         NA         pm+pt         NA           Protected Phases         3         2         1         6           Detector Phase         3         2         1         6           Switch Phase	· · · ·						
Shared Lane Traffic (%)           Lane Group Flow (vph)         187         0         764         0         220         720           Turn Type         Prot         NA         pm+pt         NA           Protected Phases         3         2         1         6           Detector Phase         3         2         1         6           Detector Phase         3         2         1         6           Switch Phase							
Lane Group Flow (vph)         187         0         764         0         220         720           Turn Type         Prot         NA         pm+pt         NA           Protected Phases         3         2         1         6           Detector Phase         3         2         1         6           Switch Phase	Heavy Vehicles (%)	1%	0%	2%	2%	1%	2%
Lane Group Flow (vph)         187         0         764         0         220         720           Turn Type         Prot         NA         pm+pt         NA           Protected Phases         3         2         1         6           Detector Phase         3         2         1         6           Switch Phase	Shared Lane Traffic (%)						
Turn TypeProtNA $pm+pt$ NAProtected Phases3216Permitted Phases3216Detector Phase3216Switch Phase		187	0	764	0	220	720
Protected Phases         3         2         1         6           Permitted Phases         6         6           Detector Phase         3         2         1         6           Switch Phase         7         6         5         6           Minimum Initial (s)         6.0         15.0         6.0         15.0           Minimum Split (s)         11.0         20.0         50.0         20.0         70.0           Total Split (s)         20.0         50.0         20.0         70.0         70.0           Total Split (%)         22.2%         55.6%         22.2%         77.8%           Yellow Time (s)         4.0         4.0         4.0         4.0           All-Red Time (s)         1.0         1.0         1.0         1.0           Lost Time Adjust (s)         0.0         0.0         0.0         0.0           Total Lost Time (s)         5.0         5.0         5.0         5.0           Lead/Lag         Lag         Lead         Lag         Lead-Lag Optimize?           Recall Mode         None         Min         None         Min           Act Effect Green (s)         13.0         37.6         52.7         52							
Permitted Phases         6           Detector Phase         3         2         1         6           Switch Phase						• •	
Detector Phase         3         2         1         6           Switch Phase         Minimum Initial (s)         6.0         15.0         6.0         15.0           Minimum Split (s)         11.0         20.0         11.0         20.0         70.0           Total Split (s)         22.2%         55.6%         22.2%         77.8%           Yellow Time (s)         4.0         4.0         4.0         4.0           All-Red Time (s)         1.0         1.0         1.0         1.0           Lost Time Adjust (s)         0.0         0.0         0.0         0.0           Total Lost Time (s)         5.0         5.0         5.0         5.0           Lead-Lag Optimize?         E         E         E           Recall Mode         None         Min         None         Min           Act Effet Green (s)         13.0         37.6         52.7         52.7           Actuated g/C Ratio         0.17         0.49         0.69         0.69           v/c Ratio         0.67         0.88         0.61         0.58           Control Delay         45.2         31.2         16.1         8.2           LOS         D         C <td< td=""><td></td><td>Ŭ</td><td></td><td>-</td><td></td><td></td><td>Ũ</td></td<>		Ŭ		-			Ũ
Switch Phase           Minimum Initial (s)         6.0         15.0         6.0         15.0           Minimum Split (s)         11.0         20.0         11.0         20.0           Total Split (s)         20.0         50.0         20.0         70.0           Total Split (%)         22.2%         55.6%         22.2%         77.8%           Yellow Time (s)         4.0         4.0         4.0         4.0           All-Red Time (s)         1.0         1.0         1.0         1.0           Lost Time Adjust (s)         0.0         0.0         0.0         0.0           Total Lost Time (s)         5.0         5.0         5.0         5.0           Lead/Lag         Lag         Lead         Lead-Lag         Lead           Lead-Lag Optimize?         Recall Mode         None         Min         None         Min           Act Effct Green (s)         13.0         37.6         52.7         52.7         52.7           Actuated g/C Ratio         0.17         0.49         0.69         0.69           v/c Ratio         0.67         0.88         0.61         0.58           Control Delay         45.2         31.2         16.1         8.2 <td></td> <td>ς</td> <td></td> <td>2</td> <td></td> <td></td> <td>6</td>		ς		2			6
Minimum Initial (s)         6.0         15.0         6.0         15.0           Minimum Split (s)         11.0         20.0         11.0         20.0           Total Split (s)         20.0         50.0         20.0         70.0           Total Split (s)         22.2%         55.6%         22.2%         77.8%           Yellow Time (s)         4.0         4.0         4.0         4.0           All-Red Time (s)         1.0         1.0         1.0         1.0           Lost Time Adjust (s)         0.0         0.0         0.0         0.0           Total Lost Time (s)         5.0         5.0         5.0         5.0           Lead/Lag         Lag         Lead         Lead         Lead           Lead-Lag Optimize?         Testifict Green (s)         13.0         37.6         52.7         52.7           Actuated g/C Ratio         0.17         0.49         0.69         0.69           v/c Ratio         0.67         0.88         0.61         0.58           Control Delay         45.2         31.2         16.1         8.2           LOS         D         C         B         A           Approach Delay         45.2         31.2<		J		Z		1	0
Minimum Split (s)         11.0         20.0         11.0         20.0           Total Split (s)         20.0         50.0         20.0         70.0           Total Split (s)         22.2%         55.6%         22.2%         77.8%           Yellow Time (s)         4.0         4.0         4.0         4.0           All-Red Time (s)         1.0         1.0         1.0         1.0           Lost Time Adjust (s)         0.0         0.0         0.0         0.0           Total Lost Time (s)         5.0         5.0         5.0         5.0           Lead/Lag         Lag         Lead         Lead/Lag         Lead           Lead-Lag Optimize?         T         0.49         0.69         0.69           v/c Ratio         0.17         0.49         0.69         0.69           v/c Ratio         0.67         0.88         0.61         0.58           Control Delay         45.2         31.2         16.1         8.2           LOS         D         C         B         A           Approach Delay         45.2         31.2         10.0           Approach LOS         D         C         B         Queue Length 50th (ft)         88		6.0		15.0		6.0	15.0
Total Split (s)         20.0         50.0         20.0         70.0           Total Split (%)         22.2%         55.6%         22.2%         77.8%           Yellow Time (s)         4.0         4.0         4.0         4.0           All-Red Time (s)         1.0         1.0         1.0         1.0           Lost Time Adjust (s)         0.0         0.0         0.0         0.0           Total Lost Time (s)         5.0         5.0         5.0         5.0           Lead/Lag         Lag         Lead         Lead           Lead-Lag Optimize?         Recall Mode         None         Min         None         Min           Act Effct Green (s)         13.0         37.6         52.7         52.7         52.7           Act uated g/C Ratio         0.17         0.49         0.69         0.69         0.69           v/c Ratio         0.67         0.88         0.61         0.58         20         0.0         0.0         0.0         0.0           Total Delay         45.2         31.2         16.1         8.2         2         LOS         D         C         B         A           Approach Delay         45.2         31.2         10.0 <td>.,</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	.,						
Total Split (%)         22.2%         55.6%         22.2%         77.8%           Yellow Time (s)         4.0         4.0         4.0         4.0         4.0           All-Red Time (s)         1.0         1.0         1.0         1.0         1.0           Lost Time Adjust (s)         0.0         0.0         0.0         0.0         0.0           Total Lost Time (s)         5.0         5.0         5.0         5.0         5.0           Lead/Lag         Lag         Lead         Lead         Lead         Lead           Lead-Lag Optimize?         Recall Mode         None         Min         None         Min           Act Effct Green (s)         13.0         37.6         52.7         52.7           Actuated g/C Ratio         0.17         0.49         0.69         0.69           v/c Ratio         0.67         0.88         0.61         0.58           Control Delay         45.2         31.2         16.1         8.2           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         45.2         31.2         16.1         8.2           LOS         D         C         B         A							
Yellow Time (s)       4.0       4.0       4.0       4.0         All-Red Time (s)       1.0       1.0       1.0       1.0         Lost Time Adjust (s)       0.0       0.0       0.0       0.0         Total Lost Time (s)       5.0       5.0       5.0       5.0         Lead-Lag Optimize?       Lead       Lead       Lead         Recall Mode       None       Min       None       Min         Act Effct Green (s)       13.0       37.6       52.7       52.7         Actuated g/C Ratio       0.17       0.49       0.69       0.69         v/c Ratio       0.67       0.88       0.61       0.58         Control Delay       45.2       31.2       16.1       8.2         Queue Delay       0.0       0.0       0.0       0.0         Total Delay       45.2       31.2       16.1       8.2         LOS       D       C       B       A         Approach Delay       45.2       31.2       10.0         Approach LOS       D       C       B       B         Queue Length 50th (ft)       88       311       33       159         Queue Length 95th (ft)							
All-Red Time (s)1.01.01.01.0Lost Time Adjust (s)0.00.00.00.0Total Lost Time (s)5.05.05.05.0Lead/LagLagLeadLeadLead-Lag Optimize?EeadEeadRecall ModeNoneMinNoneAct Effct Green (s)13.037.652.7Actuated g/C Ratio0.170.490.690.670.880.610.58Control Delay45.231.216.1Queue Delay0.00.00.0Total Delay45.231.216.1LOSDCBApproach Delay45.231.210.0Approach LOSDCBQueue Length 95th (ft)#191#60699Queue Length 95th (ft)#191#60699238Internal Link Dist (ft)1050882151Turn Bay Length (ft)33410774681519Starvation Cap Reductn0000							
Lost Time Adjust (s)         0.0         0.0         0.0         0.0           Total Lost Time (s)         5.0         5.0         5.0         5.0           Lead/Lag         Lag         Lead         Lead           Lead-Lag Optimize?         Eecall Mode         None         Min         None         Min           Act Effct Green (s)         13.0         37.6         52.7         52.7           Actuated g/C Ratio         0.17         0.49         0.69         0.69           v/c Ratio         0.67         0.88         0.61         0.58           Control Delay         45.2         31.2         16.1         8.2           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         45.2         31.2         16.1         8.2           LOS         D         C         B         A           Approach Delay         45.2         31.2         10.0           Approach LOS         D         C         B         B           Queue Length 95th (ft)         #191         #606         99         238           Internal Link Dist (ft)         1050         882         151         151	.,						
Total Lost Time (s)         5.0         5.0         5.0         5.0           Lead/Lag         Lag         Lead           Lead-Lag Optimize?         Recall Mode         None         Min         None         Min           Act Effct Green (s)         13.0         37.6         52.7         52.7         Actuated g/C Ratio         0.17         0.49         0.69         0.69           v/c Ratio         0.67         0.88         0.61         0.58         Control Delay         45.2         31.2         16.1         8.2           Queue Delay         0.0         0         0.0         0         0         0         0         0         0         0         0         0         0         0	.,						
Lead/Lag         Lag         Lead           Lead-Lag Optimize?         Recall Mode         None         Min         None         Min           Act Effct Green (s)         13.0         37.6         52.7         52.7           Actuated g/C Ratio         0.17         0.49         0.69         0.69           v/c Ratio         0.67         0.88         0.61         0.58           Control Delay         45.2         31.2         16.1         8.2           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         45.2         31.2         16.1         8.2           LOS         D         C         B         A           Approach Delay         45.2         31.2         10.0           Approach LOS         D         C         B           Queue Length 50th (ft)         88         311         33         159           Queue Length 95th (ft)         #191         #606         99         238           Internal Link Dist (ft)         1050         882         151           Turn Bay Length (ft)         334         1077         468         1519           Starvation Cap Reductn         0							
Lead-Lag Optimize?           Recall Mode         None         Min         None         Min           Act Effct Green (s)         13.0         37.6         52.7         52.7           Actuated g/C Ratio         0.17         0.49         0.69         0.69           v/c Ratio         0.67         0.88         0.61         0.58           Control Delay         45.2         31.2         16.1         8.2           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         45.2         31.2         16.1         8.2           LOS         D         C         B         A           Approach Delay         45.2         31.2         10.0           Approach LOS         D         C         B           Queue Length 50th (ft)         88         311         33         159           Queue Length 95th (ft)         #191         #606         99         238           Internal Link Dist (ft)         1050         882         151           Turn Bay Length (ft)         334         1077         468         1519           Starvation Cap Reductn         0         0         0         0         0<	Total Lost Time (s)	5.0		5.0		5.0	5.0
Lead-Lag Optimize?           Recall Mode         None         Min         None         Min           Act Effct Green (s)         13.0         37.6         52.7         52.7           Actuated g/C Ratio         0.17         0.49         0.69         0.69           v/c Ratio         0.67         0.88         0.61         0.58           Control Delay         45.2         31.2         16.1         8.2           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         45.2         31.2         16.1         8.2           LOS         D         C         B         A           Approach Delay         45.2         31.2         10.0           Approach LOS         D         C         B           Queue Length 50th (ft)         88         311         33         159           Queue Length 95th (ft)         #191         #606         99         238           Internal Link Dist (ft)         1050         882         151           Turn Bay Length (ft)         334         1077         468         1519           Starvation Cap Reductn         0         0         0         0         0<	Lead/Lag			Lag		Lead	
Recall Mode         None         Min         None         Min           Act Effct Green (s)         13.0         37.6         52.7         52.7           Actuated g/C Ratio         0.17         0.49         0.69         0.69           v/c Ratio         0.67         0.88         0.61         0.58           Control Delay         45.2         31.2         16.1         8.2           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         45.2         31.2         16.1         8.2           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         45.2         31.2         16.1         8.2           LOS         D         C         B         A           Approach Delay         45.2         31.2         10.0           Approach LOS         D         C         B           Queue Length 50th (ft)         88         311         33         159           Queue Length 95th (ft)         #191         #606         99         238           Internal Link Dist (ft)         1050         882         151           Turn Bay Length (ft)         334	Lead-Lag Optimize?						
Act Effct Green (s)13.037.652.752.7Actuated g/C Ratio0.170.490.690.69v/c Ratio0.670.880.610.58Control Delay45.231.216.18.2Queue Delay0.00.00.00.0Total Delay45.231.216.18.2LOSDCBAApproach Delay45.231.210.0Approach Delay45.231.210.0Approach Delay45.231.210.0IOSDCBQueue Length 50th (ft)8831133Queue Length 95th (ft)#191#60699238Internal Link Dist (ft)1050882151Turn Bay Length (ft)33410774681519Starvation Cap Reductn0000		None		Min		None	Min
Actuated g/C Ratio       0.17       0.49       0.69       0.69         v/c Ratio       0.67       0.88       0.61       0.58         Control Delay       45.2       31.2       16.1       8.2         Queue Delay       0.0       0.0       0.0       0.0         Total Delay       45.2       31.2       16.1       8.2         LOS       D       C       B       A         Approach Delay       45.2       31.2       10.0         Approach Delay       45.2       31.2       10.0         Approach LOS       D       C       B         Queue Length 50th (ft)       88       311       33       159         Queue Length 95th (ft)       #191       #606       99       238         Internal Link Dist (ft)       1050       882       151         Turn Bay Length (ft)       334       1077       468       1519         Starvation Cap Reductn       0       0       0       0	Act Effct Green (s)						52.7
v/c Ratio         0.67         0.88         0.61         0.58           Control Delay         45.2         31.2         16.1         8.2           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         45.2         31.2         16.1         8.2           LOS         D         C         B         A           Approach Delay         45.2         31.2         10.0           Approach Delay         45.2         31.2         10.0           Approach LOS         D         C         B           Queue Length 50th (ft)         88         311         33         159           Queue Length 95th (ft)         #191         #606         99         238           Internal Link Dist (ft)         1050         882         151           Turn Bay Length (ft)         334         1077         468         1519           Starvation Cap Reductn         0         0         0         0							
Control Delay         45.2         31.2         16.1         8.2           Queue Delay         0.0         0.0         0.0         0.0           Total Delay         45.2         31.2         16.1         8.2           LOS         D         C         B         A           Approach Delay         45.2         31.2         10.0           Approach LOS         D         C         B           Queue Length 50th (ft)         88         311         33         159           Queue Length 95th (ft)         #191         #606         99         238           Internal Link Dist (ft)         1050         882         151           Turn Bay Length (ft)         334         1077         468         1519           Starvation Cap Reductn         0         0         0         0							
Queue Delay         0.0         0.0         0.0         0.0           Total Delay         45.2         31.2         16.1         8.2           LOS         D         C         B         A           Approach Delay         45.2         31.2         10.0           Approach Delay         45.2         31.2         10.0           Approach LOS         D         C         B           Queue Length 50th (ft)         88         311         33         159           Queue Length 95th (ft)         #191         #606         99         238           Internal Link Dist (ft)         1050         882         151           Turn Bay Length (ft)         334         1077         468         1519           Starvation Cap Reductn         0         0         0         0							
Total Delay       45.2       31.2       16.1       8.2         LOS       D       C       B       A         Approach Delay       45.2       31.2       10.0         Approach LOS       D       C       B         Queue Length 50th (ft)       88       311       33       159         Queue Length 95th (ft)       #191       #606       99       238         Internal Link Dist (ft)       1050       882       151         Turn Bay Length (ft)       334       1077       468       1519         Starvation Cap Reductn       0       0       0       0							
LOS         D         C         B         A           Approach Delay         45.2         31.2         10.0           Approach LOS         D         C         B           Queue Length 50th (ft)         88         311         33         159           Queue Length 95th (ft)         #191         #606         99         238           Internal Link Dist (ft)         1050         882         151           Turn Bay Length (ft)         334         1077         468         1519           Starvation Cap Reductn         0         0         0         0	5						
Approach Delay         45.2         31.2         10.0           Approach LOS         D         C         B           Queue Length 50th (ft)         88         311         33         159           Queue Length 95th (ft)         #191         #606         99         238           Internal Link Dist (ft)         1050         882         151           Turn Bay Length (ft)         334         1077         468         1519           Starvation Cap Reductn         0         0         0         0							
Approach LOS         D         C         B           Queue Length 50th (ft)         88         311         33         159           Queue Length 95th (ft)         #191         #606         99         238           Internal Link Dist (ft)         1050         882         151           Turn Bay Length (ft)         334         1077         468         1519           Starvation Cap Reductn         0         0         0         0						В	
Queue Length 50th (ft)8831133159Queue Length 95th (ft)#191#60699238Internal Link Dist (ft)1050882151Turn Bay Length (ft)83410774681519Base Capacity (vph)33410774681519Starvation Cap Reductn0000							
Queue Length 95th (ft)         #191         #606         99         238           Internal Link Dist (ft)         1050         882         151           Turn Bay Length (ft)         334         1077         468         1519           Starvation Cap Reductn         0         0         0         0							
Internal Link Dist (ft)         1050         882         151           Turn Bay Length (ft)							
Turn Bay Length (ft)Base Capacity (vph)33410774681519Starvation Cap Reductn0000						99	
Base Capacity (vph)         334         1077         468         1519           Starvation Cap Reductn         0         0         0         0		1050		882			151
Starvation Cap Reductn 0 0 0 0							
Starvation Cap Reductn 0 0 0 0	Base Capacity (vph)	334		1077		468	1519
		0		0		0	0
	Spillback Cap Reductn	0		0		0	0

Existing PM

### Lanes and Geometrics 1: Hartford Ave (Rt. 126) & Maple Street

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Storage Cap Reductn	0		0		0	0
Reduced v/c Ratio	0.56		0.71		0.47	0.47
Intersection Summary						
Area Type:	Other					
Cycle Length: 90						
Actuated Cycle Length: 76	.1					
Natural Cycle: 60						
Control Type: Actuated-Ur	coordinated					
Maximum v/c Ratio: 0.88						
Intersection Signal Delay:	22.1			In	tersection	LOS: C
Intersection Capacity Utiliz	ation 71.2%			IC	U Level o	f Service
Analysis Period (min) 15						
# 95th percentile volume	exceeds cap	acity, qu	eue may	be longer		

Queue shown is maximum after two cycles.

Splits and Phases: 1: Hartford Ave (Rt. 126) & Maple Street



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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्भ	el 🕺	
Traffic Volume (vph)	5	20	5	660	835	5
Future Volume (vph)	5	20	5	660	835	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Satd. Flow (prot)	1624	0	0	1801	1799	0
Flt Permitted	0.989					
Satd. Flow (perm)	1624	0	0	1801	1799	0
Link Speed (mph)	30			30	30	
Link Distance (ft)	589			231	621	
Travel Time (s)	13.4			5.3	14.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Heavy Vehicles (%)	0%	0%	0%	2%	2%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	28	0	0	739	934	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize						
Intersection Capacity Utiliz	ation 54.3%			IC	U Level o	of Service A
Analysis Period (min) 15						

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Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	Y			र्स	4Î		
Traffic Volume (veh/h)	5	20	5	660	835	5	
Future Volume (Veh/h)	5	20	5	660	835	5	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Hourly flow rate (vph)	6	22	6	733	928	6	
Pedestrians	U	~~~	U	700	720	0	
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)				NULLE	NULE		
Upstream signal (ft)				231			
pX, platoon unblocked	0.62			231			
vC, conflicting volume	1676	931	934				
vC1, stage 1 conf vol	1070	731	734				
vC2, stage 2 conf vol							
vCu, unblocked vol	1782	931	934				
tC, single (s)	6.4	6.2	4.1				
tC, 2 stage (s)	0.4	0.2	4.1				
tF (s)	3.5	3.3	2.2				
p0 queue free %	89	93	99				
	69 57	326	741				
cM capacity (veh/h)							
Direction, Lane #	EB 1	NB 1	SB 1				
Volume Total	28	739	934				
Volume Left	6	6	0				
Volume Right	22	0	6				
cSH	161	741	1700				
Volume to Capacity	0.17	0.01	0.55				
Queue Length 95th (ft)	15	1	0				
Control Delay (s)	32.0	0.2	0.0				
Lane LOS	D	А					
Approach Delay (s)	32.0	0.2	0.0				
Approach LOS	D						
Intersection Summary							
Average Delay			0.6				ĺ
Intersection Capacity Utilization	ation		54.3%	IC	CU Level c	f Service	
Analysis Period (min)			15				
J							

# Volume 1: Hartford Ave (Rt. 126) & Maple Street

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Ý		1	ADR	<u> </u>	<u> </u>
Traffic Volume (vph)	140	190	685	45	70	450
Future Volume (vph)	140	190	685	45	70	450
Ideal Flow (vphpl)	140	1900	1900	1900	1900	1900
Satd. Flow (prot)	1900	1900	1720	001900	1616	1749
Flt Permitted	0.979	0	1720	0	0.102	1747
	1625	0	1720	0	173	1749
Satd. Flow (perm)	1020	No	1720		1/3	1749
Right Turn on Red		INO		No		
Satd. Flow (RTOR)	20		20			20
Link Speed (mph)	30		30			30
Link Distance (ft)	1130		962			231
Travel Time (s)	25.7		21.9			5.3
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96
Growth Factor	105%	105%	105%	105%	105%	105%
Heavy Vehicles (%)	2%	2%	6%	5%	8%	5%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	361	0	798	0	77	492
Turn Type	Perm		NA		pm+pt	NA
Protected Phases			2		1	6
Permitted Phases	3		_		6	5
Detector Phase	3		2		1	6
Switch Phase	5		2			0
Minimum Initial (s)	6.0		15.0		6.0	15.0
Minimum Split (s)	11.0		20.5		11.0	20.0
	25.0		20.5 45.0			20.0 65.0
Total Split (s)					20.0	
Total Split (%)	27.8%		50.0%		22.2%	72.2%
Yellow Time (s)	4.0		4.0		4.0	4.0
All-Red Time (s)	1.0		1.0		1.0	1.0
Lost Time Adjust (s)	0.0		0.0		0.0	0.0
Total Lost Time (s)	5.0		5.0		5.0	5.0
Lead/Lag			Lag		Lead	
Lead-Lag Optimize?			Yes		Yes	
Recall Mode	None		Min		None	Min
Act Effct Green (s)	20.1		40.2		49.8	49.8
Actuated g/C Ratio	0.25		0.50		0.62	0.62
v/c Ratio	0.88		0.92		0.33	0.45
Control Delay	55.4		38.4		9.5	9.4
Queue Delay	0.0		0.0		0.0	0.0
Total Delay	55.4		38.4		9.5	9.4
LOS	55.4 E		50.4 D		9.5 A	9.4 A
					A	
Approach Delay	55.4		38.4			9.4
Approach LOS	E		D		14	A
Queue Length 50th (ft)	181		373		14	114
Queue Length 95th (ft)	#347		#642		29	176
Internal Link Dist (ft)	1050		882			151
Turn Bay Length (ft)						
Base Capacity (vph)	408		864		380	1319
Starvation Cap Reductn	0		0		0	0
Spillback Cap Reductn	0		0		0	0

No-Build AM

## Volume 1: Hartford Ave (Rt. 126) & Maple Street

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Storage Cap Reductn	0		0		0	0
Reduced v/c Ratio	0.88		0.92		0.20	0.37
Intersection Summary						
Area Type:	Other					
Cycle Length: 90						
Actuated Cycle Length: 7	9.9					
Natural Cycle: 90						
Control Type: Actuated-U	ncoordinated					
Maximum v/c Ratio: 0.92						
Intersection Signal Delay	32.4			In	tersection	LOS: C
Intersection Capacity Utili	zation 78.6%			IC	U Level o	of Service D
Analysis Period (min) 15						
# 95th percentile volum	e exceeds cap	oacity, qu	eue may	be longer		
Ouquo shown is mavir	num aftar two	cuclos				

Queue shown is maximum after two cycles.

Splits and Phases: 1: Hartford Ave (Rt. 126) & Maple Street



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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y Y			र्च	eî 👘	
Traffic Volume (vph)	10	15	20	855	515	10
Future Volume (vph)	10	15	20	855	515	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Satd. Flow (prot)	1641	0	0	1749	1746	0
Flt Permitted	0.981			0.999		
Satd. Flow (perm)	1641	0	0	1749	1746	0
Link Speed (mph)	30			30	30	
Link Distance (ft)	589			231	621	
Travel Time (s)	13.4			5.3	14.1	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	105%	105%	105%	105%	105%	105%
Heavy Vehicles (%)	2%	0%	0%	5%	5%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	28	0	0	967	580	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	ed					
Intersection Capacity Utili	ization 74.1%			IC	CU Level	of Service I

Intersection Capacity Utilization 74.1% Analysis Period (min) 15

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Movement	EBL	EBR	NBL	NBT	• SBT	SBR
Lane Configurations	Y	LDIX	NDL	<u>اطא</u>	<u>الالا</u>	
Traffic Volume (veh/h)	10	15	20	855	515	10
Future Volume (Veh/h)	10	15	20	855	515	10
Sign Control	Stop	10	20	Free	Free	10
Grade	0%			0%	0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	11	17	22	945	569	11
Pedestrians	11	17	22	745	507	11
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
				None	None	
Median type				None	None	
Median storage veh)				231		
Upstream signal (ft)	0 57			231		
pX, platoon unblocked	0.56	574	F00			
vC, conflicting volume	1564	574	580			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol	1/1/	574	500			
vCu, unblocked vol	1614	574	580			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)						
tF (s)	3.5	3.3	2.2			
p0 queue free %	82	97	98			
cM capacity (veh/h)	62	522	1004			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	28	967	580			
Volume Left	11	22	0			
Volume Right	17	0	11			
cSH	134	1004	1700			
Volume to Capacity	0.21	0.02	0.34			
Queue Length 95th (ft)	19	2	0			
Control Delay (s)	38.8	0.6	0.0			
Lane LOS	E	А				
Approach Delay (s)	38.8	0.6	0.0			
Approach LOS	E					
Intersection Summary						
Average Delay			1.1			
Intersection Capacity Utiliza	ation		74.1%	10	CU Level d	of Service
Analysis Period (min)			15			
			15			

## Lanes and Geometrics 1: Hartford Ave (Rt. 126) & Maple Street

07/18/2018

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y		<b>1</b>		1	1
Traffic Volume (vph)	80	90	575	120	200	655
Future Volume (vph)	80	90	575	120	200	655
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Satd. Flow (prot)	1635	0	1759	0	1728	1801
Flt Permitted	0.977	Ű	1707	Ű	0.111	1001
Satd. Flow (perm)	1635	0	1759	0	202	1801
Right Turn on Red	1000	No	1707	No	202	1001
Satd. Flow (RTOR)		NO		NO		
Link Speed (mph)	30		30			30
Link Distance (ft)	1130		962			231
Travel Time (s)	25.7		21.9			5.3
Confl. Peds. (#/hr)	23.1	2	21.7			0.0
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91
Growth Factor	105%	105%	105%	105%	105%	105%
Heavy Vehicles (%)	105%	105% 0%	105% 2%	105%	105%	105% 2%
5	170	0%	Z70	Ζ70	170	Ζ70
Shared Lane Traffic (%)	10/	0	001	0	221	757
Lane Group Flow (vph)	196 Dorm	0	801	0	231	756
Turn Type	Perm		NA		pm+pt	NA
Protected Phases Permitted Phases	0		2		1	6
	8		2		6	1
Detector Phase	8		2		I	6
Switch Phase	ГО		15.0		( )	15.0
Minimum Initial (s)	5.0		15.0		6.0	15.0
Minimum Split (s)	11.0		20.0		11.0	20.0
Total Split (s)	20.0		50.0		20.0	70.0
Total Split (%)	22.2%		55.6%		22.2%	77.8%
Yellow Time (s)	4.0		4.0		4.0	4.0
All-Red Time (s)	1.0		1.0		1.0	1.0
Lost Time Adjust (s)	0.0		0.0		0.0	0.0
Total Lost Time (s)	5.0		5.0		5.0	5.0
Lead/Lag			Lag		Lead	
Lead-Lag Optimize?			Yes		Yes	
Recall Mode	None		Min		None	Min
Act Effct Green (s)	13.4		40.1		56.0	56.0
Actuated g/C Ratio	0.17		0.50		0.70	0.70
v/c Ratio	0.72		0.91		0.66	0.60
Control Delay	49.5		34.5		21.3	8.4
Queue Delay	0.0		0.0		0.0	0.0
Total Delay	49.5		34.5		21.3	8.4
LOS	D		С		С	A
Approach Delay	49.5		34.5		Ŭ	11.4
Approach LOS	D		C			В
Queue Length 50th (ft)	101		358		47	172
Queue Length 95th (ft)	#205		#653		123	259
Internal Link Dist (ft)	1050		882		120	151
Turn Bay Length (ft)	1000		002			101
Base Capacity (vph)	316		1020		437	1467
Starvation Cap Reductn	316 0		1020		437	1407
Starvation Cap Reductin	U		U		U	U

2025 No-Build PM

### Lanes and Geometrics 1: Hartford Ave (Rt. 126) & Maple Street

	-	*	ŧ	*	1	Ţ
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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Spillback Cap Reductn	0		0		0	0
Storage Cap Reductn	0		0		0	0
Reduced v/c Ratio	0.62		0.79		0.53	0.52
Intersection Summary						
Area Type:	Other					
Cycle Length: 90						
Actuated Cycle Length: 7	9.7					
Natural Cycle: 60						
Control Type: Actuated-U	ncoordinated					
Maximum v/c Ratio: 0.91						
Intersection Signal Delay:	: 24.5			Int	tersection	ILOS: C
Intersection Capacity Utili	zation 74.1%			IC	U Level c	of Service D
Analysis Period (min) 15						
# 95th percentile volum	e exceeds cap	bacity, qu	eue may	be longer		
Queue shown is maxir	num after two	cycles.				

Splits and Phases: 1: Hartford Ave (Rt. 126) & Maple Street



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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्भ	el el	
Traffic Volume (vph)	5	20	5	660	835	5
Future Volume (vph)	5	20	5	660	835	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Satd. Flow (prot)	1624	0	0	1801	1799	0
Flt Permitted	0.990					
Satd. Flow (perm)	1624	0	0	1801	1799	0
Link Speed (mph)	30			30	30	
Link Distance (ft)	589			231	621	
Travel Time (s)	13.4			5.3	14.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Growth Factor	105%	105%	105%	105%	105%	105%
Heavy Vehicles (%)	0%	0%	0%	2%	2%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	29	0	0	776	980	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize						

Control Type: Unsignalized Intersection Capacity Utilization 56.5% Analysis Period (min) 15

ICU Level of Service B

2025 No-Build PM

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Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	Y	2011		<del>ا</del>	<b>1</b>	0011	
Traffic Volume (veh/h)	5	20	5	660	835	5	
Future Volume (Veh/h)	5	20	5	660	835	5	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Hourly flow rate (vph)	6	23	6	770	974	6	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (ft)				231			
pX, platoon unblocked	0.59						
vC, conflicting volume	1759	977	980				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	1935	977	980				
tC, single (s)	6.4	6.2	4.1				
tC, 2 stage (s)							
tF (s)	3.5	3.3	2.2				
p0 queue free %	86	93	99				
cM capacity (veh/h)	43	307	712				
Direction, Lane #	EB 1	NB 1	SB 1				
Volume Total	29	776	980				_
Volume Left	6	6	0				
Volume Right	23	0	6				
cSH	136	712	1700				
Volume to Capacity	0.21	0.01	0.58				
Queue Length 95th (ft)	19	1	0				
Control Delay (s)	38.7	0.2	0.0				
Lane LOS	E	А					
Approach Delay (s)	38.7	0.2	0.0				
Approach LOS	E						
Intersection Summary							
Average Delay			0.7				
Intersection Capacity Utilizat	tion		56.5%	IC	CU Level c	of Service	
Analysis Period (min)			15				

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y	TIDI(	<u>المار</u>	HUR	<u>502</u>	<u> </u>
Traffic Volume (vph)	140	190	685	45	70	450
Future Volume (vph)	140	190	685	45	70	450
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1,00	1.00	1.00	1.00	1.00	1,00
	0.922	1.00		1.00	1.00	1.00
Frt Elle Directo etc. el			0.992		0.050	
Flt Protected	0.979	0	4700	0	0.950	4740
Satd. Flow (prot)	1625	0	1720	0	1616	1749
Flt Permitted	0.979				0.103	
Satd. Flow (perm)	1625	0	1720	0	175	1749
Right Turn on Red		No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	30		30			30
Link Distance (ft)	1130		962			231
Travel Time (s)	25.7		21.9			5.3
Adj. Flow (vph)	153	208	749	49	77	492
Lane Group Flow (vph)	361	0	798	0	77	492
Turn Type	Perm	U	NA	U	pm+pt	NA
Protected Phases	- CIIII		2		- μπ+μι 1	6
Protected Phases	3		Z		6	U
	3		n			L
Detector Phase	3		2		1	6
Switch Phase			15.0		( )	15.0
Minimum Initial (s)	6.0		15.0		6.0	15.0
Minimum Split (s)	11.0		20.5		11.0	20.0
Total Split (s)	25.0		45.0		20.0	65.0
Total Split (%)	27.8%		50.0%		22.2%	72.2%
Yellow Time (s)	4.0		4.0		4.0	4.0
All-Red Time (s)	1.0		1.5		1.0	1.0
Lost Time Adjust (s)	0.0		0.0		0.0	0.0
Total Lost Time (s)	5.0		5.5		5.0	5.0
Lead/Lag	0.0		Lag		Lead	0.0
Lead-Lag Optimize?			Yes		Yes	
Recall Mode	None		Min		None	Min
	20.1		39.7		49.8	49.8
Act Effct Green (s)						
Actuated g/C Ratio	0.25		0.50		0.62	0.62
v/c Ratio	0.88		0.94		0.33	0.45
Control Delay	55.4		40.7		9.5	9.4
Queue Delay	0.0		0.0		0.0	0.0
Total Delay	55.4		40.7		9.5	9.4
LOS	E		D		А	А
Approach Delay	55.4		40.7			9.4
Approach LOS	E		D			А
Queue Length 50th (ft)	181		378		14	114
Queue Length 95th (ft)	#347		#648		29	176
Internal Link Dist (ft)	1050		882			151
Turn Bay Length (ft)	1000		502			101
Base Capacity (vph)	408		853		380	1319
			000		300 0	
Starvation Cap Reductn	0					0
Spillback Cap Reductn	0		0		0	0

Alternative 1 AM

### Lanes and Geometrics 1: Hartford Ave (Rt. 126) & Maple Street

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Lane Group		• WBL	WBR	NBT	NBR	SBL	• SBT
Storage Cap Redu	ctn	0		0	, BR	0	0
Reduced v/c Ratio		0.88		0.94		0.20	0.37
Intersection Summ	ary						
Area Type:	0	ther					
Cycle Length: 90							
Actuated Cycle Ler	ngth: 79.9						
Natural Cycle: 90							
Control Type: Actua		ordinated					
Maximum v/c Ratio	: 0.94						
Intersection Signal	Delay: 33.4	4			In	tersection	LOS: C
Intersection Capac	ity Utilizatio	on 79.0%			IC	U Level o	f Service [
Analysis Period (m							
# 95th percentile	volume ex	ceeds cap	bacity, qu	eue may	be longer		

Queue shown is maximum after two cycles.

Splits and Phases: 1: Hartford Ave (Rt. 126) & Maple Street



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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Ϋ́			र्च	et 🗧	
Traffic Volume (vph)	10	15	20	855	515	10
Future Volume (vph)	10	15	20	855	515	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.918				0.997	
Flt Protected	0.981			0.999		
Satd. Flow (prot)	1641	0	0	1749	1746	0
Flt Permitted	0.981			0.999		
Satd. Flow (perm)	1641	0	0	1749	1746	0
Link Speed (mph)	30			30	30	
Link Distance (ft)	589			231	621	
Travel Time (s)	13.4			5.3	14.1	
Adj. Flow (vph)	11	17	22	945	569	11
Lane Group Flow (vph)	28	0	0	967	580	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type, Uncignalize						

Control Type: Unsignalized Intersection Capacity Utilization 74.1%

ICU Level of Service D

Analysis Period (min) 15

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Movement	EBL	EBR	• NBL	• NBT	SBT	SBR
Lane Configurations	Y			<u>المار</u>	1 <u>20</u>	
Traffic Volume (veh/h)	10	15	20	855	515	10
Future Volume (Veh/h)	10	15	20	855	515	10
Sign Control	Stop			Free	Free	
Grade	0%			0%	0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	11	17	22	945	569	11
Pedestrians				, 10	007	
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)						
Upstream signal (ft)				231		
pX, platoon unblocked	0.55			201		
vC, conflicting volume	1564	574	580			
vC1, stage 1 conf vol	1001	071	000			
vC2, stage 2 conf vol						
vCu, unblocked vol	1615	574	580			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	0.1	0.2	1.1			
tF (s)	3.5	3.3	2.2			
p0 queue free %	82	97	98			
cM capacity (veh/h)	62	522	1004			
Direction, Lane #	EB 1 28	NB 1 967	SB 1 580			
Volume Left	20 11	907	000			
Volume Right	17	0	11			
cSH	133	1004	1700			
Volume to Capacity	0.21	0.02	0.34			
	0.21 19	0.02	0.34			
Queue Length 95th (ft)						
Control Delay (s)	39.2	0.6	0.0			
Lane LOS	E	A	0.0			
Approach Delay (s)	39.2	0.6	0.0			
Approach LOS	E					
Intersection Summary						
Average Delay			1.1			
Intersection Capacity Utilizatio	n		74.1%	IC	CU Level c	f Service
Analysis Period (min)			15			

## Lanes and Geometrics 1: Hartford Ave (Rt. 126) & Maple Street

07/18/2018

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Ý		1	ADR	<u> </u>	<u> </u>
Traffic Volume (vph)	80	90	575	120	200	655
Future Volume (vph)	80	90 90	575	120	200	655
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Satd. Flow (prot)	1635	0	1759	0	1728	1801
Flt Permitted	0.977	0	1757	0	0.116	1001
Satd. Flow (perm)	1635	0	1759	0	211	1801
Right Turn on Red	1055	No	1757	No	211	1001
Satd. Flow (RTOR)		NU		NU		
Link Speed (mph)	30		30			30
Link Distance (ft)	1130		962			231
.,	25.7					5.3
Travel Time (s)	25.7	2	21.9			5.5
Confl. Peds. (#/hr)	0.01	2	0.01	0.01	0.01	0.01
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91
Growth Factor	105%	105%	105%	105%	105%	105%
Heavy Vehicles (%)	1%	0%	2%	2%	1%	2%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	196	0	801	0	231	756
Turn Type	Perm		NA		pm+pt	NA
Protected Phases			2		1	6
Permitted Phases	8				6	
Detector Phase	8		2		1	6
Switch Phase						
Minimum Initial (s)	5.0		15.0		6.0	15.0
Minimum Split (s)	11.0		20.5		11.0	20.0
Total Split (s)	20.0		50.0		20.0	70.0
Total Split (%)	22.2%		55.6%		22.2%	77.8%
Yellow Time (s)	4.0		4.0		4.0	4.0
All-Red Time (s)	1.0		1.5		1.0	1.0
Lost Time Adjust (s)	0.0		0.0		0.0	0.0
Total Lost Time (s)	5.0		5.5		5.0	5.0
Lead/Lag	5.0				Lead	5.0
5			Lag		Yes	
Lead-Lag Optimize?	None		Yes			Min
Recall Mode	None		Min		None	Min
Act Effct Green (s)	13.4		40.2		56.6	56.6
Actuated g/C Ratio	0.17		0.50		0.71	0.71
v/c Ratio	0.72		0.91		0.66	0.60
Control Delay	49.9		35.5		20.1	8.4
Queue Delay	0.0		0.0		0.0	0.0
Total Delay	49.9		35.5		20.1	8.4
LOS	D		D		С	А
Approach Delay	49.9		35.5			11.1
Approach LOS	D		D			В
Queue Length 50th (ft)	100		362		43	172
Queue Length 95th (ft)	#205		#659		118	259
Internal Link Dist (ft)	1050		882		110	151
Turn Bay Length (ft)	1050		002			101
Base Capacity (vph)	313		999		439	1461
Starvation Cap Reductn	0		0		0	0

Alternative 1 PM

### Lanes and Geometrics 1: Hartford Ave (Rt. 126) & Maple Street

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Spillback Cap Reductn	0		0		0	0
Storage Cap Reductn	0		0		0	0
Reduced v/c Ratio	0.63		0.80		0.53	0.52
Intersection Summary						
Area Type:	Other					
Cycle Length: 90						
Actuated Cycle Length: 8	0.2					
Natural Cycle: 70						
Control Type: Actuated-U	ncoordinated					
Maximum v/c Ratio: 0.91						
Intersection Signal Delay	: 24.8			Int	tersection	LOS: C
Intersection Capacity Utili	zation 74.5%			IC	U Level c	of Service D
Analysis Period (min) 15						
# 95th percentile volum	e exceeds cap	bacity, qu	eue may	be longer		
Queue shown is maxir	num after two	cycles.				

Splits and Phases: 1: Hartford Ave (Rt. 126) & Maple Street



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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			<del>ا</del>	el el	
Traffic Volume (vph)	5	20	5	660	835	5
Future Volume (vph)	5	20	5	660	835	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Satd. Flow (prot)	1624	0	0	1801	1799	0
Flt Permitted	0.990					
Satd. Flow (perm)	1624	0	0	1801	1799	0
Link Speed (mph)	30			30	30	
Link Distance (ft)	589			231	621	
Travel Time (s)	13.4			5.3	14.1	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Growth Factor	105%	105%	105%	105%	105%	105%
Heavy Vehicles (%)	0%	0%	0%	2%	2%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	29	0	0	776	980	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize	h					

Control Type: Unsignalized Intersection Capacity Utilization 56.5% Analysis Period (min) 15

ICU Level of Service B

Alternative 1 PM

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Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	Y	2011		<del>ا</del>	<b>1</b>	0011	
Traffic Volume (veh/h)	5	20	5	660	835	5	
Future Volume (Veh/h)	5	20	5	660	835	5	
Sign Control	Stop			Free	Free		
Grade	0%			0%	0%		
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90	
Hourly flow rate (vph)	6	23	6	770	974	6	
Pedestrians							
Lane Width (ft)							
Walking Speed (ft/s)							
Percent Blockage							
Right turn flare (veh)							
Median type				None	None		
Median storage veh)							
Upstream signal (ft)				231			
pX, platoon unblocked	0.59						
vC, conflicting volume	1759	977	980				
vC1, stage 1 conf vol							
vC2, stage 2 conf vol							
vCu, unblocked vol	1936	977	980				
tC, single (s)	6.4	6.2	4.1				
tC, 2 stage (s)							
tF (s)	3.5	3.3	2.2				
p0 queue free %	86	93	99				
cM capacity (veh/h)	43	307	712				
Direction, Lane #	EB 1	NB 1	SB 1				
Volume Total	29	776	980				
Volume Left	6	6	0				
Volume Right	23	0	6				
cSH	135	712	1700				
Volume to Capacity	0.21	0.01	0.58				
Queue Length 95th (ft)	19	1	0				
Control Delay (s)	38.7	0.2	0.0				
Lane LOS	E	А					
Approach Delay (s)	38.7	0.2	0.0				
Approach LOS	E						
Intersection Summary							
Average Delay			0.7				
Intersection Capacity Utilization	ation		56.5%	IC	CU Level o	of Service	
Analysis Period (min)			15				

## Lanes and Geometrics 1: Hartford Ave (Rt. 126) & Driveway/Maple Street

07/18/2018

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			\$			÷		٦	el 🕺	
Traffic Volume (vph)	0	0	5	140	0	190	5	685	45	70	450	0
Future Volume (vph)	0	0	5	140	0	190	5	685	45	70	450	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Satd. Flow (prot)	0	1589	0	0	1625	0	0	1718	0	1616	1749	0
Flt Permitted					0.861			0.997		0.223		
Satd. Flow (perm)	0	1589	0	0	1429	0	0	1713	0	379	1749	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		649			1130			962			231	
Travel Time (s)		14.8			25.7			21.9			5.3	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Growth Factor	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%
Heavy Vehicles (%)	2%	2%	0%	2%	2%	2%	25%	6%	5%	8%	5%	2%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	5	0	0	361	0	0	803	0	77	492	0
Turn Type		NA		Perm	NA		Perm	NA		pm+pt	NA	
Protected Phases		4			8			2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		2	2		1	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0		15.0	15.0		6.0	15.0	
Minimum Split (s)	11.0	11.0		11.0	11.0		20.5	20.5		11.0	20.0	
Total Split (s)	30.0	30.0		30.0	30.0		45.0	45.0		15.0	60.0	
Total Split (%)	33.3%	33.3%		33.3%	33.3%		50.0%	50.0%		16.7%	66.7%	
Yellow Time (s)	4.0	4.0		4.0	4.0		4.0	4.0		4.0	4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0		1.5	1.5		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0			0.0		0.0	0.0	
Total Lost Time (s)		5.0			5.0			5.5		5.0	5.0	
Lead/Lag							Lag	Lag		Lead		
Lead-Lag Optimize?							Yes	Yes		Yes		
Recall Mode	None	None		None	None		Min	Min		None	Min	
Act Effct Green (s)		23.7			23.7			39.9		50.1	50.1	
Actuated g/C Ratio		0.28			0.28			0.48		0.60	0.60	
v/c Ratio		0.01			0.90			0.99		0.23	0.47	
Control Delay		23.0			56.7			53.9		9.1	11.4	
Queue Delay		0.0			0.0			0.0		0.0	0.0	
Total Delay		23.0			56.7			53.9		9.1	11.4	
LOS		С			E			D		А	В	
Approach Delay		23.0			56.7			53.9			11.1	
Approach LOS		С			E			D			В	
Queue Length 50th (ft)		2			189			~480		16	138	
Queue Length 95th (ft)		10			#358			#719		34	210	
Internal Link Dist (ft)		569			1050			882			151	
Turn Bay Length (ft)								<i>z</i> · -				
Base Capacity (vph)		478			430			815		375	1159	
Starvation Cap Reductn		0			0			0		0	0	
Spillback Cap Reductn		0			0			0		0	0	

Alternative 2 AM

Synchro 9 Report Page 1

Lanes and	Geometri	CS			
1: Hartford	Ave (Rt.	126)	& Driveway	/Maple	Street

07/18/2018
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Storage Cap Reductn		0			0			0		0	0	
Reduced v/c Ratio		0.01			0.84			0.99		0.21	0.42	
Intersection Summary												
Area Type:	Other											
Cycle Length: 90												
Actuated Cycle Length: 83	3.8											
Natural Cycle: 90												
Control Type: Actuated-Ur	ncoordinated											
Maximum v/c Ratio: 0.99												
Intersection Signal Delay:	40.4			In	tersectior	n LOS: D						
Intersection Capacity Utiliz	zation 90.8%			IC	U Level o	of Service	E					
Analysis Period (min) 15												
<ul> <li>Volume exceeds capa</li> </ul>	city, queue is	theoretic	ally infinit	te.								
Queue shown is maxin	num after two	cycles.										
# 95th percentile volume	e exceeds cap	acity, qu	eue may	be longei	ſ.							
Queue shown is maxin	num after two	cycles.										

Splits and Phases: 1: Hartford Ave (Rt. 126) & Driveway/Maple Street

Ø1		<u>→</u> <sub>Ø4</sub>
15 s	45 s	30 s
Ø6		<b>₩</b> Ø8
60 s		30 s

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			4	el 🕴	
Traffic Volume (vph)	10	15	20	855	520	10
Future Volume (vph)	10	15	20	855	520	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Satd. Flow (prot)	1641	0	0	1749	1746	0
Flt Permitted	0.981			0.999		
Satd. Flow (perm)	1641	0	0	1749	1746	0
Link Speed (mph)	30			30	30	
Link Distance (ft)	589			231	599	
Travel Time (s)	13.4			5.3	13.6	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	105%	105%	105%	105%	105%	105%
Heavy Vehicles (%)	2%	0%	0%	5%	5%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	28	0	0	967	586	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize						

Control Type: Unsignalized Intersection Capacity Utilization 74.1% Analysis Period (min) 15

ICU Level of Service D

Alternative 2 AM

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Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			<del>ب</del> ا	4	
Traffic Volume (veh/h)	10	15	20	855	520	10
Future Volume (Veh/h)	10	15	20	855	520	10
Sign Control	Stop	10	20	Free	Free	10
Grade	0%			0%	0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	11	17	22	945	575	11
Pedestrians		17	LL	745	575	
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				NULLE	NUTE	
Upstream signal (ft)				231		
pX, platoon unblocked	0.54			231		
vC, conflicting volume	0.54 1570	580	586			
vC1, stage 1 conf vol	1370	000	000			
vC2, stage 2 conf vol vCu, unblocked vol	1628	580	586			
		6.2				
tC, single (s)	6.4	0.2	4.1			
tC, 2 stage (s)	2 5	2.2	2.2			
tF (s)	3.5	3.3	2.2			
p0 queue free %	82	97	98			
cM capacity (veh/h)	60	518	999			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	28	967	586			
Volume Left	11	22	0			
Volume Right	17	0	11			
cSH	129	999	1700			
Volume to Capacity	0.22	0.02	0.34			
Queue Length 95th (ft)	20	2	0			
Control Delay (s)	40.5	0.6	0.0			
Lane LOS	E	А				
Approach Delay (s)	40.5	0.6	0.0			
Approach LOS	E					
Intersection Summary						
Average Delay			1.1			
Intersection Capacity Utiliza	ation		74.1%	IC	CU Level d	of Service
Analysis Period (min)			15			
			15			

## Lanes and Geometrics 1: Hartford Ave (Rt. 126) & Driveway/Maple Street

07/18/2018

Lane Group         EBL         EBT         EBR         WBL         WBT         WBR         NBL         NBT         NBR         SBL         SBT           Lane Configurations         + <t< th=""><th>SBR 0 1900 0 0 No</th></t<>	SBR 0 1900 0 0 No
Traffic Volume (vph)0010800905570120200655Future Volume (vph)0010800905570120200655Ideal Flow (vphpl)190019001900190019001900190019001900190019001900Satd. Flow (prot)01589001635001759017281801	0 1900 0
Traffic Volume (vph)0010800905570120200655Future Volume (vph)0010800905570120200655Ideal Flow (vphpl)190019001900190019001900190019001900190019001900Satd. Flow (prot)01589001635001759017281801	0 1900 0
Future Volume (vph)0010800905570120200655Ideal Flow (vphpl)19001900190019001900190019001900190019001900Satd. Flow (prot)01589001635001759017281801	0 1900 0
Ideal Flow (vphpl)19001900190019001900190019001900190019001900Satd. Flow (prot)01589001635001759017281801	0
Satd. Flow (prot) 0 1589 0 0 1635 0 0 1759 0 1728 1801	0
Flt Permitted 0.844 0.995 0.249	
Satd. Flow (perm) 0 1589 0 0 1412 0 0 1751 0 453 1801	No
Right Turn on Red No No No	
Satd. Flow (RTOR)	
Link Speed (mph) 30 30 30 30	
Link Distance (ft) 649 1130 962 231	
Travel Time (s) 14.8 25.7 21.9 5.3	
Confl. Peds. (#/hr) 2 2 1	2
Peak Hour Factor 0.91 0.91 0.91 0.91 0.91 0.91 0.91 0.91	0.91
Growth Factor 105% 105% 105% 105% 105% 105% 105% 105%	105%
Heavy Vehicles (%) 2% 2% 0% 1% 2% 0% 0% 2% 2% 1% 2%	2%
Shared Lane Traffic (%)	270
Lane Group Flow (vph) 0 12 0 0 196 0 0 802 0 231 756	0
Turn Type NA Perm NA Perm NA pm+pt NA	Ū
Protected Phases 4 8 2 1 6	
Permitted Phases 4 8 2 6	
Detector Phase 4 4 8 8 2 2 1 6	
Switch Phase	
Minimum Initial (s) 6.0 6.0 6.0 6.0 15.0 15.0 5.0 15.0	
Minimum Split (s)         11.0         11.0         11.0         20.5         20.5         11.0         20.0	
Total Split (s) 20.0 20.0 20.0 20.0 50.0 20.0 70.0	
Total Split (%) 22.2% 22.2% 22.2% 22.2% 55.6% 55.6% 22.2% 77.8%	
Yellow Time (s) 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0	
All-Red Time (s) 1.0 1.0 1.0 1.0 1.5 1.5 1.0 1.0	
Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0	
Total Lost Time (s) 5.0 5.0 5.0 5.0 5.0	
Lead/Lag Lag Lead	
Lead-Lag Optimize? Yes Yes Yes	
Recall Mode None None None Min Min None Min	
Act Effct Green (s) 14.2 14.2 40.8 55.7 55.7	
Actuated g/C Ratio 0.18 0.18 0.51 0.70 0.70	
v/c Ratio 0.04 0.78 0.90 0.50 0.60	
Control Delay 30.2 56.4 33.2 8.1 8.8	
Queue Delay 0.0 0.0 0.0 0.0 0.0	
Total Delay 30.2 56.4 33.2 8.1 8.8	
LOS C E C A A	
Approach Delay 30.2 56.4 33.2 8.7	
Approach LOS C E C A	
Queue Length 50th (ft)         5         100         348         35         172	
Queue Length 95th (ft) 21 #217 #626 59 261	
Internal Link Dist (ft) 569 1050 882 151	
Turn Bay Length (ft)	
Base Capacity (vph) 302 268 987 557 1473	
Starvation Cap Reductn0000	

Alternative 2 PM

Synchro 9 Report Page 1

Lanes and	Geometr	ics			
1: Hartford	Ave (Rt.	126) 8	& Drivewa	y/Maple	Street

1: Hartford Ave (F	Rt. 126) 8	& Drive	way/N	laple	Street						0//1	8/2018
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Spillback Cap Reductn		0			0			0		0	0	
Storage Cap Reductn		0			0			0		0	0	
Reduced v/c Ratio		0.04			0.73			0.81		0.41	0.51	
Intersection Summary												
Area Type:	Other											
Cycle Length: 90												
Actuated Cycle Length: 80												
Natural Cycle: 75												
Control Type: Actuated-Un	coordinated											
Maximum v/c Ratio: 0.90												
Intersection Signal Delay: 2	23.3			In	tersection	n LOS: C						

inters	cetton Signal Delay. 23.5	Intersection EOS. C
Inters	ection Capacity Utilization 105.8%	ICU Level of Service G
Analy	rsis Period (min) 15	
# 9	5th percentile volume exceeds capacity, queue may be lor	nger.
Q	ueue shown is maximum after two cycles.	

Splits and Phases: 1: Hartford Ave (Rt. 126) & Driveway/Maple Street

Ø1		<u> </u>
20 s	50 s	20 s
Ø6		<b>↓</b> Ø8
70 s		20 s

07/18/2018

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			<del>ا</del>	el el	
Traffic Volume (vph)	5	20	5	655	835	5
Future Volume (vph)	5	20	5	655	835	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Satd. Flow (prot)	1624	0	0	1801	1799	0
Flt Permitted	0.990					
Satd. Flow (perm)	1624	0	0	1801	1799	0
Link Speed (mph)	30			30	30	
Link Distance (ft)	589			231	590	
Travel Time (s)	13.4			5.3	13.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Growth Factor	105%	105%	105%	105%	105%	105%
Heavy Vehicles (%)	0%	0%	0%	2%	2%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	29	0	0	770	980	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize						

Control Type: Unsignalized Intersection Capacity Utilization 56.5% Analysis Period (min) 15

ICU Level of Service B

Alternative 2 PM

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Movement	EBL	EBR	NBL	NBT	• SBT	SBR
Lane Configurations	Y	LDIX	NUC	र्भ	<u>الالان</u>	001
Traffic Volume (veh/h)	5	20	5	655	835	5
Future Volume (Veh/h)	5	20	5	655	835	5
Sign Control	Stop	20	Ū	Free	Free	Ū
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	6	23	6	764	974	6
Pedestrians	0	20	Ū	701	,,,,	Ŭ
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				None		
Upstream signal (ft)				231		
pX, platoon unblocked	0.60			201		
vC, conflicting volume	1753	977	980			
vC1, stage 1 conf vol	1755	711	700			
vC2, stage 2 conf vol						
vCu, unblocked vol	1922	977	980			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	U.7	0.2	<b>т.</b> 1			
tF (s)	3.5	3.3	2.2			
p0 queue free %	86	93	99			
cM capacity (veh/h)	44	307	712			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	29	770	980			
Volume Left	6	6	0			
Volume Right	23	0	6			
cSH Mahama ka Gama a'ka	138	712	1700			
Volume to Capacity	0.21	0.01	0.58			
Queue Length 95th (ft)	19	1	0			
Control Delay (s)	38.0	0.2	0.0			
Lane LOS	E	А				
Approach Delay (s)	38.0	0.2	0.0			
Approach LOS	E					
Intersection Summary						
Average Delay			0.7			
Intersection Capacity Utilization	ation		56.5%	IC	CU Level o	of Service
Analysis Period (min)			15			
J						

## Lanes and Geometrics 1: Hartford Ave (Rt. 126) & Driveway/Maple Street

07/18/2018

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		\$			र्स	1		\$		٦	el 🕴	
Traffic Volume (vph)	0	0	5	140	0	190	5	685	45	70	450	0
Future Volume (vph)	0	0	5	140	0	190	5	685	45	70	450	0
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		75	0		0	0		0
Storage Lanes	0		0	0		1	0		0	1		0
Taper Length (ft)	0			0			0			0		
Satd. Flow (prot)	0	1589	0	0	1711	1531	0	1718	0	1616	1749	0
Flt Permitted					0.754			0.997		0.265		
Satd. Flow (perm)	0	1589	0	0	1358	1531	0	1713	0	451	1749	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		649			1130			962			231	
Travel Time (s)		14.8			25.7			21.9			5.3	
Peak Hour Factor	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
Growth Factor	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%	105%
Heavy Vehicles (%)	2%	2%	0%	2%	2%	2%	25%	6%	5%	8%	5%	2%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	5	0	0	153	208	0	803	0	77	492	0
Turn Type		NA		Perm	NA	Perm	Perm	NA		pm+pt	NA	-
Protected Phases		4			8			2		1	6	
Permitted Phases	4			8		8	2			6		
Detector Phase	4	4		8	8	8	2	2		1	6	
Switch Phase												
Minimum Initial (s)	6.0	6.0		6.0	6.0	6.0	15.0	15.0		6.0	15.0	
Minimum Split (s)	11.0	11.0		11.0	11.0	11.0	20.5	20.5		11.0	20.0	
Total Split (s)	21.0	21.0		21.0	21.0	21.0	58.0	58.0		11.0	69.0	
Total Split (%)	23.3%	23.3%		23.3%	23.3%	23.3%	64.4%	64.4%		12.2%	76.7%	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	4.0	4.0		4.0	4.0	
All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.5	1.5		1.0	1.0	
Lost Time Adjust (s)		0.0			0.0	0.0		0.0		0.0	0.0	
Total Lost Time (s)		5.0			5.0	5.0		5.5		5.0	5.0	
Lead/Lag							Lag	Lag		Lead		
Lead-Lag Optimize?							Yes	Yes		Yes		
Recall Mode	None	None		None	None	None	Min	Min		None	Min	
Act Effct Green (s)		14.4			14.4	14.4		40.3		48.8	48.8	
Actuated g/C Ratio		0.19			0.19	0.19		0.54		0.66	0.66	
v/c Ratio		0.02			0.58	0.70		0.86		0.19	0.43	
Control Delay		30.4			42.7	47.2		26.3		5.2	6.8	
Queue Delay		0.0			0.0	0.0		0.0		0.0	0.0	
Total Delay		30.4			42.7	47.2		26.3		5.2	6.8	
LOS		С			D	D		С		A	A	
Approach Delay		30.4			45.3			26.3			6.6	
Approach LOS		С			D			С			A	
Queue Length 50th (ft)		2			72	100		331		11	95	
Queue Length 95th (ft)		12			#159	#222		511		23	144	
Internal Link Dist (ft)		569			1050			882		20	151	
Turn Bay Length (ft)		50,				75		502				
						10						

Alternative 3 AM

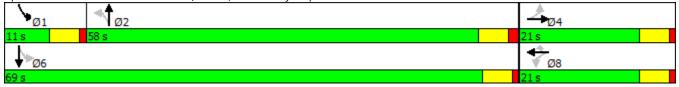
Synchro 9 Report Page 1

### Lanes and Geometrics 1: Hartford Ave (Rt. 126) & Driveway/Maple Street

07/18/2018
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Base Capacity (vph)		379			323	365		1215		400	1436	
Starvation Cap Reductn		0			0	0		0		0	0	
Spillback Cap Reductn		0			0	0		0		0	0	
Storage Cap Reductn		0			0	0		0		0	0	
Reduced v/c Ratio		0.01			0.47	0.57		0.66		0.19	0.34	
Intersection Summary												
Area Type:	Other											
Cycle Length: 90												
Actuated Cycle Length: 74	.3											
Natural Cycle: 80												
Control Type: Actuated-Un	ncoordinated											
Maximum v/c Ratio: 0.86												
Intersection Signal Delay:	23.8			In	tersectior	n LOS: C						
Intersection Capacity Utiliz	ation 78.6%			IC	CU Level o	of Service	D					
Analysis Period (min) 15												
# 95th percentile volume	exceeds cap	oacity, qu	eue may	be longe	r.							
Queue shown is maxim	num after two	cycles.										

Splits and Phases: 1: Hartford Ave (Rt. 126) & Driveway/Maple Street



	≯	*	•	1	ŧ	~
Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			4	el 🕴	
Traffic Volume (vph)	10	15	20	855	520	10
Future Volume (vph)	10	15	20	855	520	10
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Satd. Flow (prot)	1641	0	0	1749	1746	0
Flt Permitted	0.981			0.999		
Satd. Flow (perm)	1641	0	0	1749	1746	0
Link Speed (mph)	30			30	30	
Link Distance (ft)	589			231	599	
Travel Time (s)	13.4			5.3	13.6	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	105%	105%	105%	105%	105%	105%
Heavy Vehicles (%)	2%	0%	0%	5%	5%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	28	0	0	967	586	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize						

Control Type: Unsignalized Intersection Capacity Utilization 74.1% Analysis Period (min) 15

ICU Level of Service D

Alternative 3 AM

	٦	$\mathbf{i}$	•	t	Ļ	1
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			र्स	4	0.5.1
Traffic Volume (veh/h)	10	15	20	855	520	10
Future Volume (Veh/h)	10	15	20	855	520	10
Sign Control	Stop	10	20	Free	Free	10
Grade	0%			0%	0%	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Hourly flow rate (vph)	11	17	22	945	575	11
Pedestrians		17	LL	745	575	
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				NULLE	NULLE	
Upstream signal (ft)				231		
pX, platoon unblocked	0.59			231		
vC, conflicting volume	0.59	580	586			
	1570	280	080			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol	1/10	F00	F0/			
vCu, unblocked vol	1618	580	586			
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	0.5	0.0	0.0			
tF (s)	3.5	3.3	2.2			
p0 queue free %	83	97	98			
cM capacity (veh/h)	66	518	999			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	28	967	586			
Volume Left	11	22	0			
Volume Right	17	0	11			
cSH	140	999	1700			
Volume to Capacity	0.20	0.02	0.34			
Queue Length 95th (ft)	18	2	0			
Control Delay (s)	37.1	0.6	0.0			
Lane LOS	E	А				
Approach Delay (s)	37.1	0.6	0.0			
Approach LOS	E					
Intersection Summary						
Average Delay			1.0			
Intersection Capacity Utiliza	ation		74.1%	10	CU Level d	f Service
Analysis Period (min)			15			
			15			

## Lanes and Geometrics 1: Hartford Ave (Rt. 126) & Driveway/Maple Street

07/18/2018

Lane Configurations         EBI         FBI         EBR         WBI         WBI         WBI         NBI         NBI         NBI         NBI         SBI		۶	-	$\mathbf{F}$	4	•	•	•	Ť	1	1	ŧ	~
Traffic Volume (vph)         0         0         10         80         0         90         5         570         120         200         665         0           Future Volume (vph)         1900         190         1         1         0         1         1         0         1         1900         1900         190         180         180         180         180         180         180         180         180         180         180         180         180         180         180         180         180	Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Traffic Volume (vph)         0         0         10         80         0         90         5         570         120         200         655         0           Ideal Flow (vphp)         1900         1000         1000 <t< td=""><td>Lane Configurations</td><td></td><td>\$</td><td></td><td></td><td>ę</td><td>1</td><td></td><td>\$</td><td></td><td>1</td><td>eî 🕺</td><td></td></t<>	Lane Configurations		\$			ę	1		\$		1	eî 🕺	
Fulure (vph)         0         0         100         1900 <t< td=""><td></td><td>0</td><td></td><td>10</td><td>80</td><td></td><td></td><td>5</td><td></td><td>120</td><td></td><td></td><td>0</td></t<>		0		10	80			5		120			0
Ideal Flow (php)         1900         1910		0	0	10	80	0	90	5	570	120	200	655	
Storage Length (ft)       0       0       0       75       0       0       0       0         Storage Lanes       0       0       0       178       150       0       1759       0       0       1789       0       0       1789       0       0       1789       0       0       1789       0       0       1789       0       0       1789       0       0       1789       0       0       1789       0       0       1789       0       0       1789       0       0       1789       0       0       1789       0       1789       0       1789       0       1789       0       1789       0       1789       0       1789       0       0       1789       0       0       1789       0       0       1789       0       0       1789       0       0       1789       0       0       1789       0       0       1789       0       0       0       0       0       1789       0       0       0       0       1789       0       0       0       1789       0       0       0       1789       0       0       0       1786       0 <t< td=""><td></td><td>1900</td><td>1900</td><td>1900</td><td>1900</td><td>1900</td><td>1900</td><td>1900</td><td>1900</td><td>1900</td><td>1900</td><td>1900</td><td>1900</td></t<>		1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Lanes         0         0         0         1         0         0         1         0         0           Taper Length (ft)         0         1589         0         0         1728         1561         0         1759         0         1728         1801         0           FIP Fermited         0.750         0.995         0.280         5341         1801         0         1589         0         0         1364         1518         0         1751         0         509         1801         0           Statl. Flow (perm)         0         1589         0         0         1364         1518         0         1751         0         509         1801         0           Statl. Flow (perm)         0         134         1518         0         1751         0         509         1803         105         1051         1051         1051         1051         1051         1051         1051         1051         1051         1051         1051         1051         1051         1051         1051         1051         1055         1055         1055         1055         1055         1055         1055         1055         1055         1055         1		0		0	0		75	0		0	0		0
Taper Length (th)         0         0         1738         100         1759         0         1728         1801         0           Satd. Flow (prot)         0         1589         0         0         1364         1518         0         1759         0         509         1801         0           Satd. Flow (perm)         0         1589         0         0         1364         1518         0         1751         0         509         1801         0           Satd. Flow (perm)         0         1589         0         0         1364         1518         0         1750         0         509         1801         0           Satd. Flow (perm)         30         -         30         -         30         -         30         -         30         -         30         -         20         -         21         1         22         231         -         53         -         22         27         21         27         28         28         105%         105%         105%         105%         105%         105%         105%         105%         105%         105%         105%         105%         105%         105%         105%		0		0	0		1	0		0	1		0
Said. How (pcrot)       0       1589       0       0       1728       1561       0       1759       0       1728       1801       0         FIt Permitted       0.750       0.995       0.280       0.280       0       100       1751       0       509       1801       0         Right Turn on Red       No       No       No       1751       0       509       1801       0         Right Turn on Red       No       130       30       30       30       30       30       30       30       1751       0       1059       101       0       1059       1059       101       0       105       1050       1050       1055       1050       1050       1050       1055       1050       1050       1055       1050       1050       1050       1050       1050       1050       1050       1050       1050       <											0		
FIT Permitted       0.750       0.995       0.280       V         Satd. Flow (perm)       0       1589       0       0       1364       1518       0       171       0       509       1801       0No         Satd. Flow (RTOR)       No       No       No       No       No       No       No       No       No         Link Speed (mph)       30       30       30       30       30       231       Tavel (mph)       5.3       Conft. Peck (wh)       91       0.	1 0 . ,	0	1589	0	0	1728	1561	0	1759	0	1728	1801	0
Sald. Flow (perm)       0       1589       0       0       1364       1518       0       1751       0       509       1801       0         Right Turn on Red       No       No       No       No       No       No       No       No         Link Distance (ft)       649       1130       962       231       Tavel Time (S)       14.8       25.7       21.9       231       Tavel Time (S)       14.8       25.7       2.19       25.3       221.9       105%						0.750			0.995		0.280		
Right Turn on RedNoNoNoNoSaid. Flow (RTOR)	Satd. Flow (perm)	0	1589	0	0	1364	1518	0	1751	0	509	1801	0
Said. How (RTOR)         Jink Distance (II)         30         30         30         30         30           Link Distance (II)         649         1130         962         221         5.3           Confl. Peds. (#/n)         2         2         1         5.3         2           Confl. Peds. (#/n)         2         2         1         2         2         1         2         2         2         2         2         2         2         2         2         1         2         2         2         2         2         2         1         2         2         2         2         2         1         2         2         1         2         2         1         0         0         9         0         9         0         10 <td></td>													
Link Speed (mph)         30         30         30         30         30           Link Distance (tt)         649         1130         962         231           Confi. Peds. (#hn)         2         2.19         5.3           Confi. Peds. (#hn)         2         2         1         2.29           Peak Hour Factor         0.91													
			30			30			30			30	
$\begin{array}{c c c c c c c c c c c c c c c c c c c $													
Confl. Peds. (#/hr)         2         2         1         2         2           Peak Hour Factor         0.91	<b>、</b> ,												
Peak Hour Factor         0.91	.,	2					2	1					2
Growth Factor         105%         105         100         100         105         15.0         15.0         15.0         15.0	· · ·		0.91	0.91	0.91	0.91		0.91	0.91	0.91	0.91	0.91	
Heavy Vehicles (%)       2%       0%       0%       2%       2%       1%       0       0       0       0       0       231       756       0       0       0       0       0       0       0       2%       2%       1%       0       <													
Shared Lane Traffic (%)       Lane Group Flow (vph)       0       12       0       0       92       104       0       802       0       231       756       0         Turn Type       NA       Perm       NA       Perm       NA       Perm       NA       protested Phases       4       8       2       1       6         Permitted Phases       4       4       8       8       2       5       1       6         Switch Phase       4       4       8       8       8       2       5       1       6         Switch Phase       11.0       11.0       11.0       11.0       20.5       20.5       11.0       20.0         Minimum Initial (\$)       6.0       6.0       6.0       6.0       6.0       6.0       6.0       15.0       15.0       15.0       15.0         Minimum Initial (\$)       11.0       11.0       11.0       11.0       11.0       12.0       17.0       <													
Lane Group Flow (vph)         0         12         0         0         92         104         0         802         0         231         756         0           Turn Type         NA         Perm         NA         Perm         NA         Perm         NA         pm+pt         NA           Protected Phases         4         8         8         2         1         6           Detector Phase         4         4         8         8         2         2         1         6           Winhimum Initial (s)         6.0         6.0         6.0         11.0         11.0         20.5         11.0         20.0           Total Split (s)         17.0         17.0         17.0         17.0         62.0         62.0         11.0         73.0           Total Split (s)         17.0         17.0         17.0         17.0         62.0         62.0         11.0         73.0           Yelow Time (s)         4.0 </td <td></td>													
Turn TypeNAPermNAPermPermNApm+ptNAProtected Phases488216Permitted Phases44882216Detector Phase44882216Switch Phase44882216Switch Phase	· · · ·	0	12	0	0	92	104	0	802	0	231	756	0
Protected Phases         4         8         2         1         6           Permitted Phases         4         4         8         8         2         6           Detector Phase         4         4         8         8         2         2         1         6           Switch Phase         Minimu Initial (s)         6.0         6.0         6.0         15.0         15.0         5.0         15.0           Minimu Initial (s)         6.0         6.0         6.0         11.0         11.0         20.0         10         73.0           Total Split (s)         17.0         17.0         17.0         17.0         62.0         62.0         11.0         73.0           Total Split (%)         18.9%         18.9%         18.9%         18.9%         68.9%         68.9%         12.2%         81.1%           Yeltow Time (s)         4.0         4	1 1 1	-		-						-			
Permitted Phases         4         8         8         2         6           Detector Phase         4         4         8         8         2         2         1         6           Switch Phase											· ·		
Detector Phase         4         4         8         8         8         2         2         1         6           Switch Phase		4			8		8	2			6		
Switch Phase         Minimum Initial (s)         6.0         6.0         6.0         6.0         15.0         15.0         15.0           Minimum Split (s)         11.0         11.0         11.0         11.0         20.5         20.5         11.0         20.0           Total Split (s)         17.0         17.0         17.0         17.0         62.0         62.0         11.0         20.0           Total Split (s)         18.9%         18.9%         18.9%         18.9%         68.9%         68.9%         12.2%         81.1%           Yellow Time (s)         4.0	Detector Phase	4	4		8	8		2	2		1	6	
Minimum Initial (s)         6.0         6.0         6.0         6.0         15.0         15.0         5.0         15.0           Minimum Split (s)         11.0         11.0         11.0         11.0         11.0         20.5         11.0         20.0           Total Split (s)         17.0         17.0         17.0         17.0         62.0         62.0         11.0         73.0           Total Split (%)         18.9%         18.9%         18.9%         18.9%         68.9%         68.9%         62.0         11.0         73.0           Total Split (%)         18.9%         18.9%         18.9%         18.9%         68.9%         68.9%         62.0         11.0         73.0           Yellow Time (s)         4.0         4.0         4.0         4.0         4.0         4.0         4.0         4.0           All-Red Time (s)         1.0 </td <td></td>													
Minimum Split (s)         11.0         11.0         11.0         11.0         11.0         20.5         20.5         11.0         20.0           Total Split (s)         17.0         17.0         17.0         17.0         17.0         62.0         62.0         11.0         73.0           Total Split (s)         18.9%         18.9%         18.9%         18.9%         68.9%         68.9%         68.9%         4.0         4.0         4.0           All-Red Time (s)         1.0	Minimum Initial (s)	6.0	6.0		6.0	6.0	6.0	15.0	15.0		5.0	15.0	
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	. ,	11.0	11.0		11.0	11.0	11.0	20.5	20.5		11.0	20.0	
Total Split (%)18.9%18.9%18.9%18.9%18.9%68.9%68.9%68.9%12.2%81.1%Yellow Time (s)4.04.04.04.04.04.04.04.04.04.0All-Red Time (s)1.01.01.01.01.51.51.01.01.0Lost Time Adjust (s)0.00.00.00.00.00.00.0Total Lost Time (s)5.05.05.05.55.05.0Lead/LagEadLagLeadLeadLead-Lag Optimize?YesYesYesRecall ModeNoneNoneNoneNoneMinMinActuated g/C Ratio0.140.140.140.570.750.78v/c Ratio0.050.470.470.810.460.54Control Delay32.140.339.620.06.36.2LoSCDBAAApproach Delay32.139.920.06.2Approach LOSCDBAAQueue Length 50th (ft)5374226426128Queue Length 95th (ft)229910843349219		17.0	17.0		17.0	17.0	17.0	62.0			11.0		
Yellow Time (s)4.04.04.04.04.04.04.04.0All-Red Time (s)1.01.01.01.01.01.51.51.01.0Lost Time Adjust (s)0.00.00.00.00.00.00.0Total Lost Time (s)5.05.05.05.55.05.0Lead/Lag		18.9%	18.9%		18.9%	18.9%	18.9%	68.9%	68.9%		12.2%	81.1%	
Lost Time Adjust (s)         0.0         0.0         0.0         0.0         0.0         0.0           Total Lost Time (s)         5.0         5.0         5.0         5.0         5.0         5.0           Lead/Lag         Lead         Lag         Lag         Lead           Lead-Lag Optimize?         Yes         Yes         Yes           Recall Mode         None         None         None         Min         Min         None         Min           Act Effct Green (s)         9.8         9.8         9.8         38.3         50.9         52.7           Actuated g/C Ratio         0.14         0.14         0.14         0.57         0.75         0.78           v/c Ratio         0.05         0.47         0.47         0.81         0.46         0.54           Control Delay         32.1         40.3         39.6         20.0         6.3         6.2           Queue Delay         0.0         0.0         0.0         0.0         0.0         0.0         0.0         0.0           Total Delay         32.1         40.3         39.6         20.0         6.3         6.2           LOS         C         D         D         B		4.0	4.0		4.0	4.0	4.0	4.0	4.0		4.0	4.0	
Total Lost Time (s) $5.0$ $5.0$ $5.0$ $5.0$ $5.0$ $5.0$ $5.0$ Lead/LagLagLagLagLagLeadLead-Lag Optimize?YesYesYesRecall ModeNoneNoneNoneNoneMinMinAct Effct Green (s)9.89.89.838.3 $50.9$ $52.7$ Actuated g/C Ratio0.140.140.140.570.750.78v/c Ratio0.050.470.470.810.460.54Control Delay32.140.339.620.06.36.2Queue Delay0.00.00.00.00.00.00.0Total Delay32.140.339.620.06.36.2LOSCDDBAAApproach Delay32.139.920.06.2Approach LOSCDBAAQueue Length 50th (ft)5374226426128Queue Length 95th (ft)229910843349219	All-Red Time (s)	1.0	1.0		1.0	1.0	1.0	1.5	1.5		1.0	1.0	
Lead/Lag         Lag         Lag         Lag         Lead           Lead-Lag Optimize?         Yes         Yes         Yes           Recall Mode         None         None         None         None         Min         Min         None         Min           Act Effct Green (s)         9.8         9.8         9.8         38.3         50.9         52.7           Actuated g/C Ratio         0.14         0.14         0.14         0.57         0.75         0.78           V/c Ratio         0.05         0.47         0.47         0.81         0.46         0.54           Queue Delay         32.1         40.3         39.6         20.0         6.3         6.2           Queue Delay         0.0         0.0         0.0         0.0         0.0         0.0         0.0           Total Delay         32.1         40.3         39.6         20.0         6.3         6.2           LOS         C         D         D         B         A         A           Approach Delay         32.1         39.9         20.0         6.2         A           Approach LOS         C         D         B         A         A         A <t< td=""><td>Lost Time Adjust (s)</td><td></td><td>0.0</td><td></td><td></td><td>0.0</td><td>0.0</td><td></td><td>0.0</td><td></td><td>0.0</td><td>0.0</td><td></td></t<>	Lost Time Adjust (s)		0.0			0.0	0.0		0.0		0.0	0.0	
Lead/Lag         Lag         Lag         Lag         Lead           Lead-Lag Optimize?         Yes         Yes         Yes           Recall Mode         None         None         None         None         Min         Min         None         Min           Act Effct Green (s)         9.8         9.8         9.8         38.3         50.9         52.7           Actuated g/C Ratio         0.14         0.14         0.14         0.57         0.75         0.78           V/c Ratio         0.05         0.47         0.47         0.81         0.46         0.54           Queue Delay         32.1         40.3         39.6         20.0         6.3         6.2           Queue Delay         0.0         0.0         0.0         0.0         0.0         0.0         0.0           Total Delay         32.1         40.3         39.6         20.0         6.3         6.2           LOS         C         D         D         B         A         A           Approach Delay         32.1         39.9         20.0         6.2         A           Approach LOS         C         D         B         A         A         A <t< td=""><td>Total Lost Time (s)</td><td></td><td>5.0</td><td></td><td></td><td>5.0</td><td>5.0</td><td></td><td>5.5</td><td></td><td>5.0</td><td>5.0</td><td></td></t<>	Total Lost Time (s)		5.0			5.0	5.0		5.5		5.0	5.0	
Lead-Lag Optimize?         Yes         Yes         Yes           Recall Mode         None         None         None         None         Min         Min         Min           Act Effct Green (s)         9.8         9.8         9.8         38.3         50.9         52.7           Actuated g/C Ratio         0.14         0.14         0.14         0.57         0.75         0.78           v/c Ratio         0.05         0.47         0.47         0.81         0.46         0.54           Control Delay         32.1         40.3         39.6         20.0         6.3         6.2           Queue Delay         0.0         0.0         0.0         0.0         0.0         0.0           Total Delay         32.1         40.3         39.6         20.0         6.3         6.2           LOS         C         D         D         B         A         A           Approach Delay         32.1         39.9         20.0         6.2         A           Approach LOS         C         D         B         A         A         A         A         A         A         A         A         A         A         A         A <t< td=""><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>Lag</td><td>Lag</td><td></td><td>Lead</td><td></td><td></td></t<>								Lag	Lag		Lead		
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v/c Ratio0.050.470.470.810.460.54Control Delay32.140.339.620.06.36.2Queue Delay0.00.00.00.00.00.0Total Delay32.140.339.620.06.36.2LOSCDDBAAApproach Delay32.139.920.06.2LOSCDBAAApproach Delay32.139.920.06.2Queue Length 50th (ft)5374226426128Queue Length 95th (ft)229910843349219			0.14			0.14	0.14		0.57		0.75	0.78	
Queue Delay         0.0         0.0         0.0         0.0         0.0         0.0           Total Delay         32.1         40.3         39.6         20.0         6.3         6.2           LOS         C         D         D         B         A         A           Approach Delay         32.1         39.9         20.0         6.2           Queue Length Soth (ft)         5         37         42         264         26         128           Queue Length 95th (ft)         22         99         108         433         49         219			0.05			0.47	0.47		0.81		0.46	0.54	
Total Delay       32.1       40.3       39.6       20.0       6.3       6.2         LOS       C       D       D       B       A       A         Approach Delay       32.1       39.9       20.0       6.2         Approach Delay       32.1       39.9       20.0       6.2         Approach LOS       C       D       B       A         Queue Length 50th (ft)       5       37       42       264       26       128         Queue Length 95th (ft)       22       99       108       433       49       219	Control Delay		32.1			40.3	39.6		20.0		6.3	6.2	
Total Delay       32.1       40.3       39.6       20.0       6.3       6.2         LOS       C       D       D       B       A       A         Approach Delay       32.1       39.9       20.0       6.2         Approach Delay       32.1       39.9       20.0       6.2         Approach LOS       C       D       B       A         Queue Length 50th (ft)       5       37       42       264       26       128         Queue Length 95th (ft)       22       99       108       433       49       219	Queue Delay		0.0			0.0					0.0		
LOS         C         D         D         B         A         A           Approach Delay         32.1         39.9         20.0         6.2           Approach LOS         C         D         B         A           Queue Length 50th (ft)         5         37         42         264         26         128           Queue Length 95th (ft)         22         99         108         433         49         219						40.3					6.3		
Approach Delay         32.1         39.9         20.0         6.2           Approach LOS         C         D         B         A           Queue Length 50th (ft)         5         37         42         264         26         128           Queue Length 95th (ft)         22         99         108         433         49         219			С								А		
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Queue Length 50th (ft)5374226426128Queue Length 95th (ft)229910843349219	11 3												
Queue Length 95th (ft)         22         99         108         433         49         219							42				26		
5 (7													

Alternative 3 PM

Synchro 9 Report Page 1

Lanes and	Geome	trics			
1: Hartford	Ave (R	t. 126)	& Drivewa	y/Maple	Street

07/18/2018	
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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Turn Bay Length (ft)						75						
Base Capacity (vph)		308			264	294		1428		501	1646	
Starvation Cap Reductn		0			0	0		0		0	0	
Spillback Cap Reductn		0			0	0		0		0	0	
Storage Cap Reductn		0			0	0		0		0	0	
Reduced v/c Ratio		0.04			0.35	0.35		0.56		0.46	0.46	
Intersection Summary												
Area Type:	Other											
Cycle Length: 90												
Actuated Cycle Length: 67	.7											
Natural Cycle: 60												
Control Type: Actuated-Un	coordinated											
Maximum v/c Ratio: 0.81												
Intersection Signal Delay: 7				In	tersectior	n LOS: B						
Intersection Capacity Utiliz	ation 99.9%			IC	CU Level o	of Service	F					
Analysis Period (min) 15												

Splits and Phases: 1: Hartford Ave (Rt. 126) & Driveway/Maple Street

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11s 62s	17 s
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73 s	17 s

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Lane Group	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	Y			<del>ا</del>	el el	
Traffic Volume (vph)	5	20	5	655	835	5
Future Volume (vph)	5	20	5	655	835	5
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Satd. Flow (prot)	1624	0	0	1801	1799	0
Flt Permitted	0.990					
Satd. Flow (perm)	1624	0	0	1801	1799	0
Link Speed (mph)	30			30	30	
Link Distance (ft)	589			231	590	
Travel Time (s)	13.4			5.3	13.4	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Growth Factor	105%	105%	105%	105%	105%	105%
Heavy Vehicles (%)	0%	0%	0%	2%	2%	0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	29	0	0	770	980	0
Sign Control	Stop			Free	Free	
Intersection Summary						
Area Type:	Other					
Control Type: Unsignalize						

Control Type: Unsignalized Intersection Capacity Utilization 56.5% Analysis Period (min) 15

ICU Level of Service B

Alternative 3 PM

	٦	$\mathbf{i}$	•	t	Ļ	1
Movement	EBL	EBR	NBL	NBT	• SBT	SBR
Lane Configurations	Y	LDIX	NDL	र्भ	<u>الالان</u>	JUN
Traffic Volume (veh/h)	5	20	5	655	835	5
Future Volume (Veh/h)	5	20	5	655	835	5
Sign Control	Stop	20	5	Free	Free	0
Grade	0%			0%	0%	
Peak Hour Factor	0.90	0.90	0.90	0.90	0.90	0.90
Hourly flow rate (vph)	6	23	6	764	974	6
Pedestrians	0	25	0	704	774	0
Lane Width (ft)						
Walking Speed (ft/s)						
Percent Blockage						
Right turn flare (veh)						
Median type				None	None	
Median storage veh)				None	None	
				231		
Upstream signal (ft)	0.65			231		
pX, platoon unblocked		977	000			
vC, conflicting volume	1753	977	980			
vC1, stage 1 conf vol						
vC2, stage 2 conf vol	1001	077	980			
vCu, unblocked vol	1891	977				
tC, single (s)	6.4	6.2	4.1			
tC, 2 stage (s)	2.5	2.2	0.0			
tF (s)	3.5	3.3	2.2			
p0 queue free %	88	93	99			
cM capacity (veh/h)	50	307	712			
Direction, Lane #	EB 1	NB 1	SB 1			
Volume Total	29	770	980			
Volume Left	6	6	0			
Volume Right	23	0	6			
cSH	149	712	1700			
Volume to Capacity	0.19	0.01	0.58			
Queue Length 95th (ft)	17	1	0			
Control Delay (s)	35.0	0.2	0.0			
Lane LOS	D	А				
Approach Delay (s)	35.0	0.2	0.0			
Approach LOS	D					
Intersection Summary						
Average Delay			0.7			
Intersection Capacity Utilizat	tion		56.5%	10	CU Level d	of Service
Analysis Period (min)			15	IC.		
			10			

Appendix E: MassDOT Highway Division's Project Development Process

#### **Overview of the Project Development Process**

Transportation decision-making is complex and can be influenced by legislative mandates, environmental regulations, financial limitations, agency programmatic commitments, and partnering opportunities. Decision-makers and reviewing agencies, when consulted early and often throughout the project development process, can ensure that all participants understand the potential impact these factors can have on project implementation. Project development is the process that takes a transportation improvement from concept through construction.

The MassDOT Highway Division has developed a comprehensive project development process which is contained in Chapter 2 of the *MassDOT Highway Division's Project Development and Design Guide*. The eight-step process covers a range of activities extending from identification of a project need, through completion of a set of finished contract plans, to construction of the project. The sequence of decisions made through the project development process progressively narrows the project focus and, ultimately, leads to a project that addresses the identified needs. The descriptions provided below are focused on the process for a highway project, but the same basic process will need to be followed for non-highway projects as well.

#### 1. Needs Identification

For each of the locations at which an improvement is to be implemented, MassDOT leads an effort to define the problem, establishes project goals and objectives, and defines the scope of the planning needed for implementation. To that end, it has to complete a Project Need Form (PNF), which states in general terms the deficiencies or needs related to the transportation facility or location. The PNF documents the problems and explains why corrective action is needed. For this study, the information defining the need for the project will be drawn primarily, perhaps exclusively, from the present report. Also, at this point in the process, MassDOT meets with potential participants, such as the Metropolitan Planning Organization (MPO) and community members, to allow for an informal review of the project.

The PNF is reviewed by the MassDOT Highway Division district office whose jurisdiction includes the location of the proposed project. MassDOT also sends the PNF to the MPO, for informational purposes. The outcome of this step determines whether the project requires further planning, whether it is already well supported by prior planning studies, and, therefore, whether it is ready to move forward into the design phase, or whether it should be dismissed from further consideration.

#### 2. Planning

This phase will likely not be required for the implementation of the improvements proposed in this planning study, as this planning report should constitute the outcome of this step. However, in general, the purpose of this implementation step is for the project proponent to identify issues, impacts, and approvals that may need to be obtained, so that the subsequent design and permitting processes are understood.

The level of planning needed will vary widely, based on the complexity of the project. Typical tasks include: define the existing context, confirm project need, establish goals and objectives, initiate public outreach, define the project, collect data, develop and analyze alternatives, make recommendations, and provide documentation. Likely outcomes include consensus on the project definition to enable it to move forward into environmental documentation (if needed) and design, or a recommendation to delay the project or dismiss it from further consideration.

#### 3. Project Initiation

At this point in the process, the proponent, MassDOT Highway Division, fills out a Project Initiation Form (PIF) for each improvement, which is reviewed by its Project Review Committee (PRC) and the MPO. The PRC is composed of the Chief Engineer, each District Highway Director, and representatives of the Project Management, Environmental, Planning, Right-of-Way, Traffic, and Bridge departments, and the MassDOT Federal Aid Program Office (FAPO). The PIF documents the project type and description, summarizes the project planning process, identifies likely funding and project management responsibility, and defines a plan for interagency and public participation. First the PRC reviews and evaluates the proposed project based on the MassDOT's statewide priorities and criteria. If the result is positive, MassDOT Highway Division moves the project forward to the design phase, and to programming review by the MPO. The PRC may provide a Project Management Plan to define roles and responsibilities for subsequent steps. The MPO review includes project evaluation based on the MPO's regional priorities and criteria. The MPO may assign project evaluation criteria score, a Transportation Improvement Program (TIP) year, a tentative project category, and a tentative funding category.

#### 4. Environmental Permitting, Design, and Right-of-Way Process

This step has four distinct but closely integrated elements: public outreach, environmental documentation and permitting (if required), design, and right-of-way acquisition (if required). The outcome of this step is a fully designed and permitted project ready for construction. However, a project does not have to be fully designed in order for the MPO to program it in the TIP. The sections below provide more detailed information on the four elements of this step of the project development process.

#### Public Outreach

Continued public outreach in the design and environmental process is essential to maintain public support for the project and to seek meaningful input on the design elements. The public outreach is often in the form of required public hearings, but can also include less formal dialogues with those interested in and affected by a proposed project.

#### Environmental Documentation and Permitting

The project proponent, in coordination with the Environmental Services section of the MassDOT Highway Division, will be responsible for identifying and complying with all applicable federal, state, and local environmental laws and requirements. This includes determining the appropriate project category for both the Massachusetts Environmental Protection Act (MEPA) and the National Environmental Protection Act (NEPA). Environmental documentation and permitting is often completed in conjunction with the **Preliminary Design** phase described below.

#### Design

There are three major phases of design. The first is **Preliminary Design**, which is also referred to as the 25-percent submission. The major components of this phase include full survey of the project area, preparation of base plans, development of basic geometric layout, development of preliminary cost estimates, and submission of a functional design report. Preliminary Design, although not required to, is often completed in conjunction with the Environmental Documentation and Permitting. The next phase is **Final Design**, which is also referred to as the 75-percent and 100-percent submission. The major components of this phase include preparation of a subsurface exploratory plan (if required), coordination of utility relocations, development of traffic management plans through construction zones, development of final cost estimates, and refinement and finalization of the construction plans. Once Final Design is complete, a full set of **Plans, Specifications, and Estimates (PS&E)** is developed for the project.

#### Right-of-Way Acquisition

A separate set of Right-of-Way plans are required for any project that requires land acquisition or easements. The plans must identify the existing and proposed layout lines, easements, property lines, names of property owners, and the dimensions and areas of estimated takings and easements.

#### 5. Programming (Identification of Funding)

Programming, which typically begins during the design phase, can actually occur at any time during the process, from planning to design. In this step, which is distinct from project initiation, the proponent requests that the MPO place the project in the region's Transportation Improvement Program (TIP). The proponent requesting the project's listing on the TIP can be the community or it can be one of the MPO member agencies (the Regional Planning Agency, MassDOT, and the Regional Transit Authority). The MPO then considers the project in terms of state and regional needs, evaluation criteria, and compliance with the regional Transportation Plan and decides whether to place it in the draft TIP for public review and then in the final TIP.

#### 6. Procurement

Following project design and programming of a highway project, the MassDOT Highway Division publishes a request for proposals. It then reviews the bids and awards the contract to the qualified bidder with the lowest bid.

#### 7. Construction

After a construction contract is awarded, MassDOT Highway Division and the contractor develop a public participation plan and a management plan for the construction process.

#### 8. Project Assessment

The purpose of this step is to receive constituents' comments on the project development process and the project's design elements. MassDOT Highway Division can apply what is learned in this process to future projects.

### **Project Development Schematic Timetable**

Description	Schedule Influence	Typical Duration
Step I: Problem/Need/Opportunity	The Project Need Form has been	1 to 3 months
<b>Identification</b> The proponent completes a Project	developed so that it can be prepared	
Need Form (PNF). This form is then reviewed by	quickly by the proponent, including any	
the MassDOT District office which provides	supporting data that is readily available.	
guidance to the proponent on the subsequent steps	The District office shall return comments	
of the process.	to the proponent within one month of	
F	PNF submission.	
Step II: Planning	For some projects, no planning beyond	Project Planning
Project planning can range from agreement that	preparation of the Project Need Form is	Report: 3 to 24+
the problem should be addressed through a clear	required. Some projects require a	months
solution to a detailed analysis of alternatives and	planning study centered on specific	
their impacts.	project issues associated with the	
	proposed solution or a narrow family of	
	alternatives. More complex projects will	
	likely require a detailed alternatives	
	analysis.	
Step III: Project Initiation	The PIF includes refinement of the	1 to 4 months
The proponent prepares and submits a Project	preliminary information contained in the	
Initiation Form (PIF) and a Transportation	PNF. Additional information	
Evaluation Criteria (TEC) form in this step. The	summarizing the results of the planning	
PIF and TEC are informally reviewed by the	process, such as the Project Planning	
Metropolitan Planning Organization (MPO) and	Report, are included with the PIF and	
MassDOT District office, and formally reviewed	TEC. The schedule is determined by PRC	
by the PRC.	staff review (dependent on project	
	complexity) and meeting schedule.	
Step IV: Design, Environmental, and Right of	The schedule for this step is dependent	3  to  48 +  months
Way	upon the size of the project and the	
The proponent completes the project design.	complexity of the design, permitting, and	
Concurrently, the proponent completes necessary	right-of-way issues. Design review by the	
environmental permitting analyses and files	MassDOT district and appropriate	
applications for permits. Any right of way needed	sections is completed in this step.	
for the project is identified and the acquisition		
process begins.		
Step V: Programming	The schedule for this step is subject to	3 to $12+$ months
The MPO considers the project in terms of its	each MPO's programming cycle and	
regional priorities and determines whether or not	meeting schedule. It is also possible that	
to include the project in the draft Regional	the MPO will not include a project in its	
Transportation Improvement Program (TIP)	Draft TIP based on its review and	
which is then made available for public comment.	approval procedures.	
The TIP includes a project description and		
funding source.		
Step VI: Procurement The project is advertised	Administration of competing projects can	1 to 12 months
for construction and a contract awarded.	influence the advertising schedule.	24.60.1
Step VII: Construction The construction process	The duration for this step is entirely	3  to  60 +  months
is initiated including public notification and any	dependent upon project complexity and	
anticipated public involvement. Construction	phasing.	
continues to project completion.		1 1
Step VIII: Project Assessment The construction	The duration for this step is dependent	1 month
period is complete and project elements and	upon the proponent's approach to this	
processes are evaluated on a voluntary basis. Source: MassDOT Highway Division Project Deve	step and any follow-up required.	

Source: MassDOT Highway Division Project Development and Design Guide